

Research

Recharge-area Nuclear Waste Repository in Southeastern Sweden

Demonstration of Hydrogeologic Siting Concepts
and Techniques

Clifford I. Voss
Alden M. Provost

November 2001

SKI perspective

Background and objective

An important issue in the site selection process are a recharge area (generally an inland location) vs. a discharge area (generally a coastal location). According to SKI, this problem has not yet been thoroughly investigated by SKB, and consequently SKI initiated the present research project to clarify the implications of this factor.

Results

The results from this study indicate that, if dilution mechanisms are disregarded, it may be advantageous to site the repository in a recharge area since one thus may expect longer pathways and travel times through the geosphere. This in turn, implies more time for radioactive decay as well as other retardation mechanisms.

To achieve such long paths and travel times, radionuclide transport need to take place along regional flow paths. Local scale variations in the topography, combined with the fractures and fracture zones that characterise the Swedish bedrock, may however imply that certain locations within a recharge area have as short flowpaths and travel times as a discharge area. Nevertheless, the authors argue that the upstream recharge sites (inland locations) offer a greater chance of achieving long times and paths than do downstream discharge areas (coastal locations), where times and paths are expected to be short in any case.

Effects on SKI's activities

With this research project, SKI has pointed at an important factor in repository siting. SKB has up till now considered this factor to be of less importance, but in view of the results from this study, SKB ought to include it in their coming investigations.

Project information

Fritz Kautsky, Björn Dverstorp (to June, 2001) and Eva Simic (from September, 2001) have been responsible for this project at SKI.

SKI ref.: 14.9-000152/00034 and 14.9-000152/01267

SKI-perspektiv

Bakgrund och syfte

En viktig fråga i valet av slutförvarsplats är betydelsen av in- respektive utströmningsområden och inlands- respektive kustlägen. Enligt SKI har denna fråga hittills inte utretts tillräckligt väl av SKB. SKI initierade därför detta forskningsprojekt med syftet att få en större inblick i implikationerna av denna faktor.

Resultat

Resultaten från denna modellstudie indikerar att om man bortser från utspädningsmekanismer, så kan det vara fördelaktigt att lokalisera slutförvaret i ett inströmningsområde, eftersom detta innebär längre transportvägar och transporttider genom geosfären. Detta i sin tur innebär mer tid för radioaktiv avklingning samt andra fördröjande mekanismer, men kräver att radionuklidtransporten sker utmed regionala strömningsvägar.

Det finns dock en viss osäkerhet i huruvida inströmningsområdena verkligen har så långa flödesvägar som man skulle kunna förvänta sig. Lokala variationer i topografin i kombination med de sprickor och sprickzoner som karaktäriserar den svenska berggrunden kan nämligen innebära att vissa platser inom ett inströmningsområde har lika korta flödesvägar som ett utströmningsområde. Trots denna osäkerhet resonerar författarna som så att det är större chans att uppnå de långa transporttider och transportvägar som är fördelaktiga för en slutförvarsplats om man väljer att placera slutförvaret i ett inströmningsområde.

Effekt på SKI:s verksamhet

Med detta forskningsprojekt har SKI visat på en viktig faktor i valet av slutförvarsplats. SKB har hittills inte ansett denna faktor ha någon större betydelse men bör ta hänsyn till den i sina fortsatta utredningar.

Projektinformation

SKI:s handläggare har varit Fritz Kautsky, Björn Dverstorp (t.o.m. juni 2001) och Eva Simic (fr.o.m. september 2001).

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Abstract

Nuclear waste repositories located in regional ground-water recharge ('upstream') areas may provide the safety advantage that potentially released radionuclides would have long *travel time* and *path length*, and large *path volume*, within the bedrock before reaching the biosphere. Nuclear waste repositories located in ground-water discharge ('downstream') areas likely have much shorter *travel time* and *path length* and smaller *path volume*. Because most coastal areas are near the primary discharge areas for regional ground-water flow, coastal repositories may have a lower hydrogeologic safety margin than 'upstream' repositories located inland.

Advantageous recharge-area sites may be located through careful use of regional three-dimensional, variable-density, ground-water modeling. Because of normal limitations of site-characterization programs in heterogeneous bedrock environments, the hydrogeologic structure and properties of the bedrock will generally remain unknown at the spatial scales required for the model analysis, and a number of alternative bedrock descriptions are equally likely. Model simulations need to be carried out for the full range of possible descriptions. The favorable sites are those that perform well for all of the modeled bedrock descriptions. Structural heterogeneities in the bedrock and local undulations in water-table topography, at a scale finer than considered by a given model, also may cause some locations in favored inland areas to have very short flow paths (of only hundreds of meters) and short travel times, compromising the long times and paths (of many kilometers) predicted by the analysis for these sites. However, in the absence of more detailed modeling, the favored upstream sites offer a greater chance of achieving long times and paths than do downstream discharge areas, where times and paths are expected to be short regardless of the level of detail included in the model.

As an example of this siting approach, potential repository sites are evaluated using a three-dimensional, variable-density flow and solute transport model of southeastern Sweden under present interglacial conditions. The analysis considers four structural models of the bedrock that represent the possible range of regional anisotropy in permeability. Results indicate that potential repository sites at Hultsfred and another comparison site have *travel times* ten times or longer than sites at Simpevarp and Oskarshamn, and at worst, have *travel times* equivalent to the latter sites. Potential repository sites at Simpevarp and Oskarshamn have *flow paths* of less than 3 kilometers (lower values cannot be resolved by the current model), while the Hultsfred and comparison sites have *path lengths* ranging from 25 kilometers to 130 kilometers, and much greater *flow path volumes*. Given the uncertainties in bedrock structure, thus, it is more likely that the Hultsfred site will provide greater *flow path*, *travel time*, and *flow path volume* than the coastal Simpevarp and Oskarshamn sites, and may therefore be preferable in terms of the hydrogeologic safety margin. Additionally, there are other repository locations in southeastern Sweden, such as the comparison site, that improve substantially on the Hultsfred site.

Three techniques are introduced for illuminating important aspects of the three-dimensional flow field in a numerical model. (1) A technique for tracing the *flow paths* and *flow path volume* from potential repository sites in a three-dimensional variable-density ground-water model involves a solute transport simulation that fills the flow tube passing through the repository with a non-zero

concentration. (2) *Recharge areas* may be mapped in the model by means of a transport simulation that delineates where inflow occurs to the top surface of the model. (3) *Travel times* may be determined in a 'return-flow time' transport simulation, in which the flow field is reversed and the solute undergoes a zero-order production rate of one per year.

Introduction

Safety of a KBS3-type high-level nuclear waste repository in the crystalline bedrock of Sweden depends on two barriers: 1- the engineered barrier consisting of the spent nuclear fuel rods encased in metal canisters that are deposited in holes bored in the bottom of drifts and surrounded by a bentonite shroud, and, 2- the geologic barrier consisting of the crystalline bedrock both immediately surrounding the repository drifts and extending in all directions from the repository. Proper functioning of the engineered barrier system depends on the geomechanical stability and geochemical properties of the subsurface environment. If radionuclide leakage should occur from the engineered barrier system, then only the geologic barrier can provide safety. Two of the key factors that determine the long-term radiologic safety of a nuclear waste repository provided by the geologic barrier are the *travel time* and *path length* traveled before released radionuclides reach the biosphere (see, for example, the SR-97 study (SKB, 1999)). According to analysis of this system along stream tubes, these two factors, together with the flow-wetted surface area, control the retarding function of the repository rock for all nuclides (except non-sorbing, long-lived nuclides that escape irrespective of hydrogeologic properties). The *travel time* and *path length* depend entirely on the hydrogeologic situation of the repository within the ground-water-flow field. These are a safety factors that can be controlled by judicious selection of a site. With all factors considered, it is advantageous for repository safety to select a site that provides the longest possible *travel time* and *flow path*.

The importance of two factors described above was identified on the basis of one-dimensional analysis of the equations describing water flow, transport of decaying solutes, and radionuclide retardation by sorption and diffusion into the rock matrix (e.g., see Andersson and others, 1998, and Andersson, 1999). However, in three spatial dimensions, it may be speculated that the volume of rock that is encompassed by a radioactive plume emanating from a repository also may be an important factor; larger rock volumes provide greater potential for retardation. Thus, three-dimensional analysis may find that the total volume of rock included along flow paths from a repository is an additional key factor. Larger *flow path volumes* would provide more fracture surface area for sorption and matrix diffusion processes. The *flow path volume* also depends on the hydrogeologic situation of the repository and so there are three factors that may be maximized through appropriate selection of the site.

In most ground-water systems, the longest *travel times* and *flow paths* generally would be found for repositories located in primary recharge areas that recharge large-scale ground-water flow systems. There also are present many small-scale recharge-discharge systems, the waters of which occupy the shallower portions of the subsurface. For example, small shallow systems may occur between hillsides and adjoining valleys. However, there are often recharge-discharge systems that extend across entire regional aquifer systems, the waters of which occupy the deepest portions of the subsurface (e.g., see Toth (1963)). A repository located within the recharge area for such a deep regional system would have the longest *travel time*, *path length*, and *path volume* for escaping radionuclides that is possible in the entire region. On the other hand, a repository located near the discharge area would have relatively short *travel time*, *path length*, and *path volume*,

reducing the safety margin. Thus, near-coastal repository locations, which are always near the discharge end of local and regional ground-water systems in the Fennoscandian shield, may reduce the safety margin in comparison with an inland location in a major recharge area. It is possible, for example, that repositories in discharge areas would have *travel times* of hundreds of years and *path lengths* of hundreds of meters, whereas repositories near major recharge areas would have *travel times* of hundreds of thousands of years and *path lengths* of tens of kilometers. One objective, therefore, for repository siting may be 'upstream location in a regional-scale ground-water system'. For application of this objective, it would be desirable to have maps of ground-water recharge and discharge areas and of *travel times* from all possible underground locations to aid in repository site selection. Some aspects of this concept have been considered earlier (Leijon, 1998) but have not been applied quantitatively in Fennoscandia. The U.S. Geological Survey (USGS), in cooperation with the Swedish Nuclear Power Inspectorate (SKI), completed the present study concerning the above-mentioned siting concepts and techniques for finding advantageous locations through use of numerical ground-water modeling.

Three circumstances that may complicate the achievement of this objective are as follows.

1- Because of the heterogeneity of the Fennoscandian shield bedrock, which contains variable lithology as well as fractures and fracture zones at all spatial scales, it may be possible that potentially long *flow paths* are 'short-circuited'. Permeable structures in the rock may gather flows and discharge them locally, near to their recharge areas. Fundamentally, this possibility means that primary recharge areas, those with long paths, may be more difficult to locate in such bedrock. Despite this, primary recharge areas do exist somewhere within the ground-water system, although their spatial uniformity, continuity and lateral extent may not be great enough in which to site a repository.

2- Undulations in the topography of the water table can create small, closed, shallow flow systems that may occur even in the vicinity of a major recharge area at the upstream end of a regional flow system. If the repository were placed inadvertently within the local system in this area rather than within the regional system, *flow paths* would be relatively short. Local flow systems may also cause some discharge to occur from otherwise regional systems by blocking passage of the regional ground-water flow.

The combination of heterogeneity and water table undulation may make it difficult to find a repository location within a primary recharge area. A careful analysis is required to bound the possible effects of various types of heterogeneities and undulations on the flow field in a particular region while seeking advantageous recharge area sites. The approach would be to define a variety of simple models that capture the general range of heterogeneity and water-table variation that is likely in the region of interest. Locations then are sought with long *flow paths* and *travel times* irrespective of which bedrock and water table model is assumed. These are the locations that are the most reliable for 'upstream' siting of the repository.

3- The present ground-water systems in the Fennoscandian shield are not perpetual. Climate changes, such as permafrost and glaciation, as well as isostatic rebound of the

crust and sea-level regression, will strongly affect the ground-water-flow fields; recharge and discharge areas will not necessarily remain stationary throughout the glacial cycle, and the transport of subsurface contaminants may be profoundly affected by transient flow patterns. The present-day (2001) flow field is representative only of interglacial periods. Safe siting of the repository must, therefore, consider both the climate-change-driven flow field changes and the present-day flow field. The topic of climate-change-driven flow is not considered further in the present work, but coupled climate-change and ground-water-flow analyses such as Provost and others (1998) and Boulton and others (1995) may lend some insight into matters that need to be considered.

The present study provides specific results concerning the ground-water flow system in southeastern Sweden, but all of the results presented herein deal with evaluation of the first circumstance discussed above and should be considered as a demonstration of concepts and techniques for recharge-area siting of nuclear waste repositories in Sweden. The present work defines simple models that capture the general range of heterogeneity that is likely in the region by taking into account the effect of extensive bedrock fracturing on the anisotropy of the effective regional hydraulic properties of the ground-water system. Consideration of the effect of surface topography is limited to a resolution of water-table variation on a horizontal spatial scale ranging between about 1.5 and 10 km because the analyses presented are based on relatively coarse models (with mesh spacing between 1.5 and 10 km) run on a personal desktop computer. More detailed spatial analyses would require use of a much larger computer, particularly for the three-dimensional simulations. Only the ground-water flow field based on present-day climatic conditions is considered.

The topics considered here are twofold. First, the advantage of upstream locations in regional flow systems is illustrated in two-dimensional cross sections. Second, a three-dimensional, variable-density, ground-water flow and solute transport simulation of southeastern Sweden is presented for simple models of regional bedrock permeability that represent the range of possible anisotropy. For each bedrock model, recharge and discharge areas are mapped, and *path length*, *travel time* and *path volume* are determined for repositories in various locations. Repository locations considered include Oskarshamn, Simpevarp and Hultsfred, which are potential sites for the Swedish high-level repository, and a 'comparison' site that performs well irrespective of the bedrock model. A general ranking of repository locations in southeastern Sweden in terms of the 'upstream location' objective is demonstrated. Further, techniques for using a variable-density flow and transport code to trace *flow paths*, to delineate *recharge areas*, and to map *travel times*, are demonstrated in this report.

Recharge-Area and Discharge-Area Repositories

In siting an underground nuclear waste repository, the possibility of a release of radioactive contaminants to the subsurface environment must be considered. The longer the *travel time* (and *path length*) that radionuclides travel through the subsurface, and the greater the volume of rock that they must travel through, the less radioactive they are when they reach the biosphere. Long paths and large rock volumes provide the best opportunity for escaping radionuclides to diffuse into and sorb onto the less hydraulically conductive portions of the rock. Thus, all other factors considered, it is prudent to locate repositories in areas where water has just entered the ground and must travel a long distance and a long time before returning to the surface.

Toth and Sheng (1994) formalized this idea by calling it the “Recharge Area Concept”. By definition, water enters the subsurface in ground-water *recharge areas* and exits to the surface in ground-water *discharge areas*. Toth & Sheng (1996) concluded that siting repositories according to the “Recharge Area Concept” has the following advantages:

- Contaminants released in recharge areas have a longer *travel path* and *return-flow time* (the time it takes them to travel from the repository to the surface) than contaminants released in discharge areas within the same ground-water body.
- Predictions of return-flow times and other performance parameters made for recharge areas are less sensitive to uncertainties than predictions made for discharge areas.
- Site identification, screening, and selection are simplified because flow is generally downward.

Toth and Sheng (1996) reached these conclusions partly on the basis of a model of ground-water flow through a hypothetical crystalline rock basin. Their model had the following features:

- two-dimensional, steady-state, ground-water flow
- sinusoidal surface topography
- stratified hydraulic conductivity that decreases with depth
- a single, linear, highly-conductive “fault” that passes (at various angles) through or near repositories located in the recharge and discharge areas
- basin properties representative of the Canadian shield, which is similar qualitatively to the Fennoscandian shield in terms of hydrogeology

The “Recharge Area Concept” may be regarded as one possible criterion on which to base the siting of nuclear waste repositories in the Fennoscandian shield. From a hydrogeologic point of view, a regional recharge area should provide the longest *flow paths* and *travel times* in a region, irrespective of other hydrogeologic considerations. However, because of limitations in characterizing rock heterogeneity and water-table undulations in the region, there is inevitably some uncertainty in the calculation of the flow path and travel time from such a location. Indeed, there is some uncertainty as to whether a given location is in reality within a regional-scale flow system or in a local small-scale flow system. As in the approach of Toth & Sheng (1996), these

effects may be demonstrated by modeling simple systems that capture the essential characteristics of fractured crystalline rock environments in the Fennoscandian shield.

APPROACH

Following Toth & Sheng (1996), the advantages of recharge-area repositories over discharge-area repositories may be demonstrated by generating maps of *return-flow time* based on numerical simulation of ground-water flow and solute transport. The modeling approach used here utilizes the USGS (U.S. Geological Survey) ground-water flow and solute transport code SUTRA (Voss, 1984), here, in two spatial dimensions, and a USGS preprocessor based on the graphical user interface package ArgusONE (Voss and others, 1997). This package allows rapid model construction and modification, calculation of steady-state or transient ground-water flow and solute transport, and calculation of steady-state *return-flow time* over the entire model domain.

Models are constructed by imposing the appropriate head distribution at the top boundary of a rectangular model domain to approximate the effects of the topography of the water table. In contrast to the approach of Toth and Sheng (1996), *return-flow times* are calculated by *reversing* the flow field (through the boundary conditions) and specifying a constant, uniform production of “solute”, the concentration of which represents ground-water age, with a concentration of zero at recharge. A zero-order solute production of one per year is used. Reversing the boundary conditions, in this case, simply requires multiplication of all specified hydraulic heads by ‘-1’.

Return Flow Times without Fracture Zones

Simulation results for the following case are illustrated in *Figure 1*.

- Model domain is 20-km long by 4-km deep
- Constant-density fluid
- No flow across the lateral and bottom boundaries
- Specified hydraulic head varies sinusoidally along the top boundary, changing by 400 m over a distance of 20 km (mean head gradient = 0.02)
- Uniform (i.e., not stratified) rock matrix properties: hydraulic conductivity = 10^{-10} m/s; porosity = 0.01, in order to compare directly with results of Toth & Sheng (1996) for the Canadian shield
- No explicitly represented fracture zones

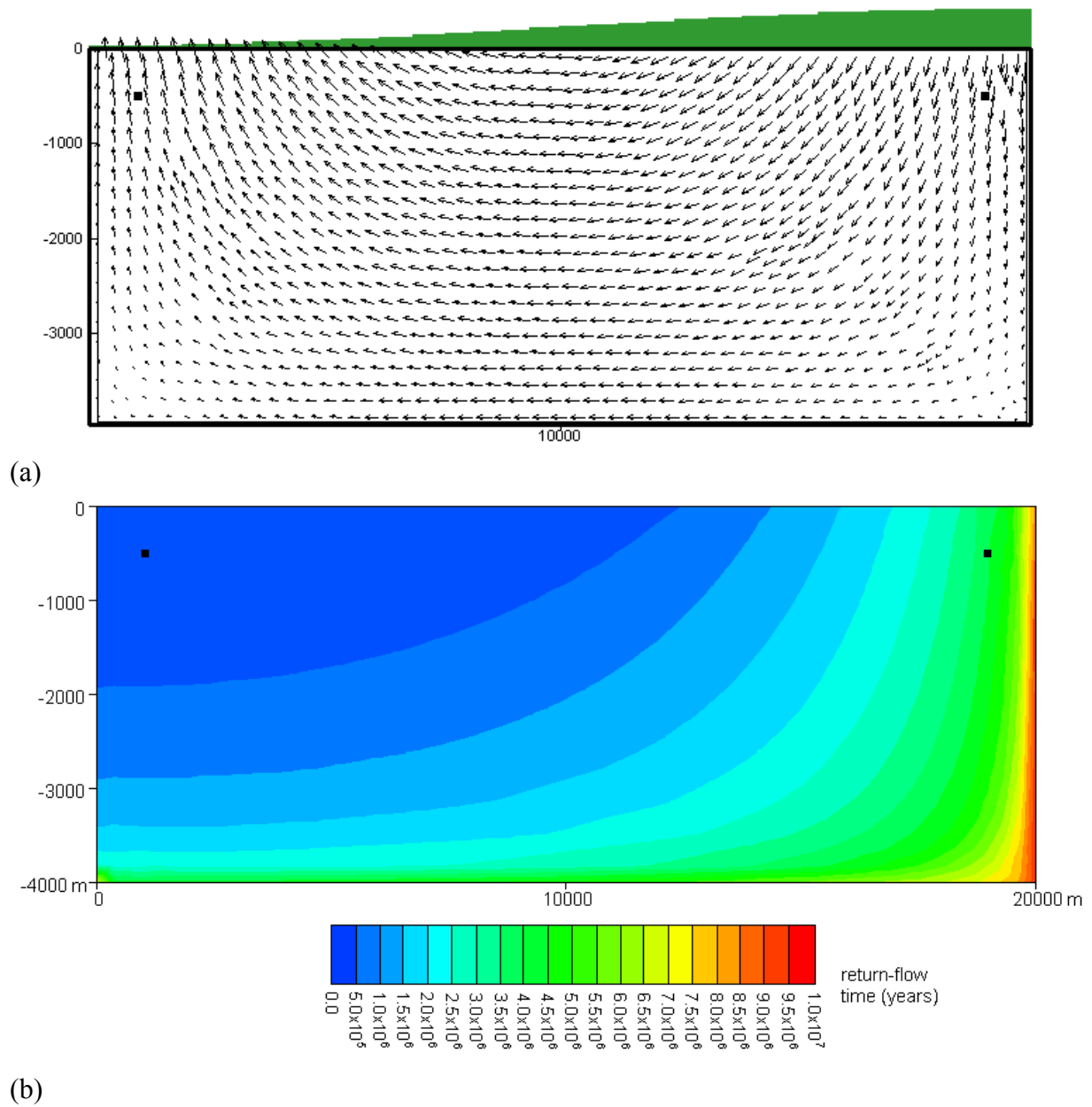


Figure 1. Simple case with no fracture zones: (a) sinusoidal water-table topography (*green*) and velocity vectors for the regional flow field; (b) map of *return-flow times*. Black squares denote hypothetical repositories at 500 m depth in the recharge (right) and discharge (left) areas. Vertical exaggeration is 2 times (2x). Elevations in (m).

Under the imposed head gradient, water enters the ground along the right side of the top boundary (the regional recharge area) and exits along the left side of the top boundary (the regional discharge area). For contaminants released from a hypothetical repository located 1 km from the right-hand boundary and 500 m below the top of the model (in the recharge area), the simulated *return-flow time* is about 3.9 million years (Ma). This value agrees reasonably well with the value of 3.4 Ma obtained by Toth & Sheng (1996) using a different method to compute ground-water age. For a repository sited in the *discharge* area (1 km in from the left-hand boundary and 500 m below the top of the model domain), the *return-flow time* is only 0.11 Ma.

Overall, for nearby locations in the region of the recharge-area repository at 500 m depth, *return-flow times* are all approximately 5 Ma. For nearby locations in the region of the discharge-area repository at 500 m depth, *return-flow times* all are approximately 0.1 Ma. Thus, repository locations in the recharge area have *return-flow times* about 50 times greater than repository locations in the discharge area, and *path lengths* about 40 times greater. All other factors considered, longer ground-water systems would have proportionally longer paths and *return-flow times*. For example, in a 60-km-long system, the *return-flow time* would be about 150 times greater and the *path length* 120 times greater than for the recharge-area repository than for the discharge-area repository. This relative time advantage could be even greater in variable-density fluid systems like the Fennoscandian shield, where deep saline flow systems have velocities thousands of times lower.

In the Fennoscandian shield, hydraulic conductivities for the rock matrix (considered unfractured at the kilometer scale) are similar to the case considered, whereas porosities are typically one to two orders of magnitude lower than those assumed in this example (Voss & Andersson, 1991). In this case, all else being equal, the *return-flow times* would be reduced by one to two orders of magnitude. If the rock matrix in Sweden were considered to include the kilometer-scale network of fractures zones assumed by Voss & Andersson (1991), the rock matrix would have one to two orders of magnitude higher conductivity, and return-flow times would be a total of two to four orders of magnitude shorter than in this example. Hydraulic gradients, which also affect return-flow times, may be higher or lower than in this example, depending on location. Ratios of *travel times* and *path lengths* for the recharge- and discharge-area repositories would remain the same.

Return Flow Times with Local Topographic Variation and Conductive Fracture Zones

The presence of undulations in topography or highly conductive fracture zones may affect the location of primary recharge areas in ground-water systems. The distribution of return-flow times for the case in which a valley separates the recharge area from the regional discharge area are shown in *Figure 2*. Additionally, a nonlinear conductive fracture zone (or a series of connected fracture zone segments) passes through the repository and outcrops in the valley. The fracture zone is 100 times more permeable and 10 times more porous than the rock matrix. For this water-

table configuration, but in the absence of the fracture zone, an additional simulation (not shown) gives a return-flow time in the general vicinity of the recharge-area repository of 3.7 Ma, similar to that obtained in the case shown in *Figure 1*. The local valley causes a local ground-water system to develop above the deeper regional system. The local system is visible as the curved, darker blue area in the upper right portion of the model (*Figure 2*). The local system is present even without fracture zones. Water in the shallow system discharges near the local valley, shortening travel time and path length for repositories located within this shallow ground-water body. The upstream repository, when there is no fracture zone, is within the regional flow system.

However, the fracture zone disturbs and offsets the upstream portion of the dividing line between the shallow and deep systems, whereas the downstream portion is not strongly affected. If the fracture zone is present, the shallow flow system captures discharge from the repository because the upstream boundary of the shallow system is shifted just beyond the repository location. With the fracture zone in place, the *return-flow time* decreases by approximately a factor of 10.

Overall, for most locations within the shallow local flow system (i.e., within the darker blue area) the *return-flow time* is on the order of 0.1 Ma or less, whereas for repositories situated in the recharge area of the regional flow system (to the right of the darker blue area), *return-flow time* is on the order of 5 Ma or greater. Repositories located within the local system have little advantage (in terms of *return-flow time*) over repositories located at 500 m depth within the regional discharge area, which also have return-flow times on the order of 0.1 Ma.

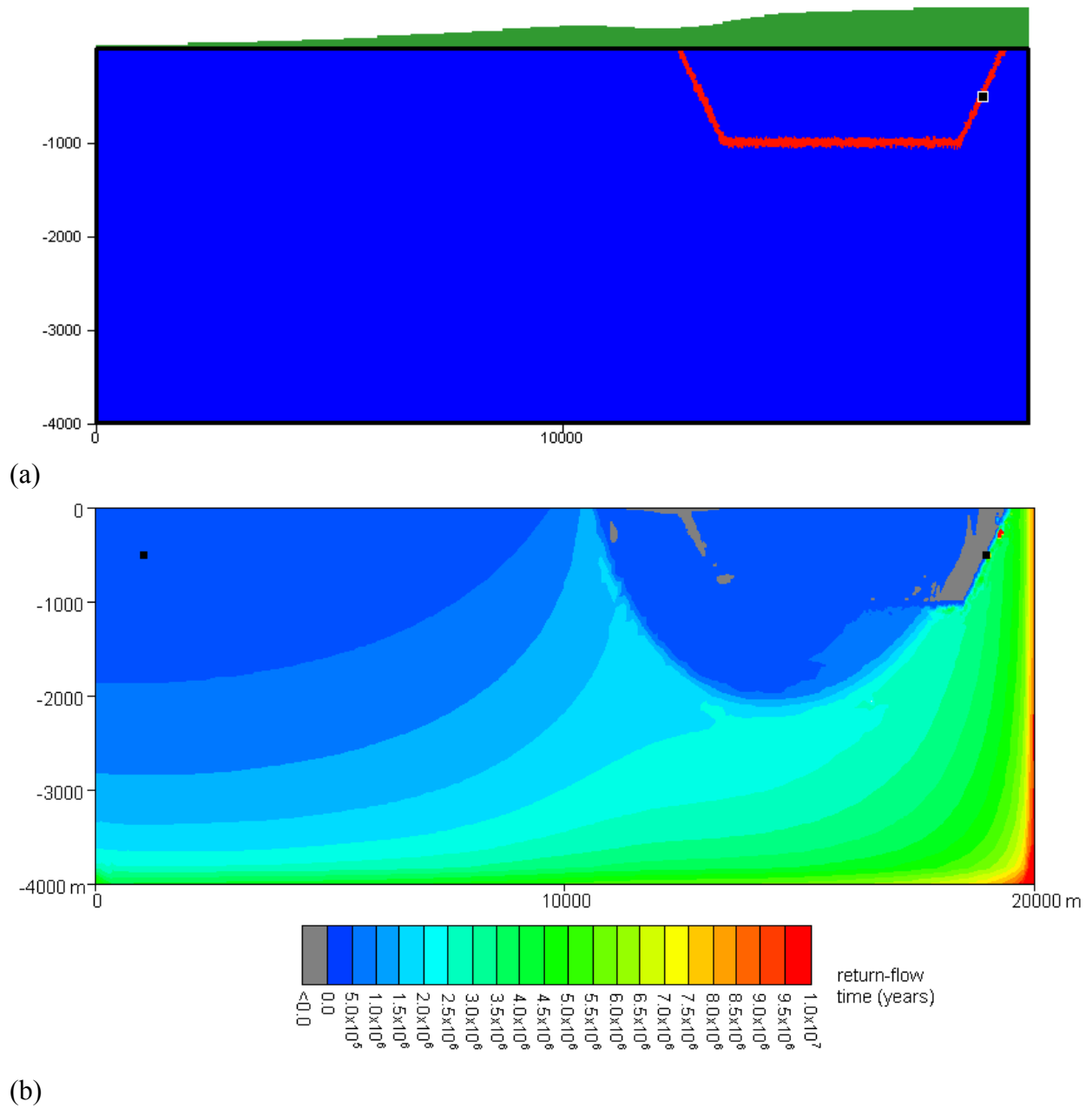


Figure 2. Variation on the base case – fracture zone and valley present: (a) surface topography (green) and fracture zone (red); (b) map of return-flow time. Black squares denotes hypothetical repositories in the recharge and discharge areas. Vertical exaggeration is 2x. Elevations in (m). Gray color indicates zones of numerical instability.

Return Flow Times that Illustrate Relations in Southeastern Sweden

Return-flow times within a two-dimensional (2D) vertical cross section of southeastern Sweden that passes approximately through the repository sites considered later in the three-dimensional (3D) model is shown in *Figure 3*. This 2D result is shown only for illustrative purposes as it entails major simplifications in comparison with the 3D model described later, and it represents only one possible model of the bedrock properties. Because of these simplifications, the ***return-flow time*** distribution shown here only should be considered as indicative of its spatial distribution; furthermore, absolute values may be quite different from the 3D model with the same bedrock properties.

The hydrogeologic properties represented by this 2D model are those of the ***10:1 Case*** (10:1 horizontal-to-vertical anisotropy in the permeability) described later in reference to the 3D model (see the section titled “**Representation of the bedrock fabric**”). Here, only constant-density fluid flow is considered. Further, the model reaches a depth of only 3 km, although the 3D model described later reaches 10 km depth. The bottom of the 2D model may be considered as being approximately at the depth of shield brines, which act approximately as a no-flow boundary to the freshwater flows above. The topography along the top of this 2D model is exactly that of the 3D model (see *Figure 4*) along the line of section shown in *Figure 3*. The repository locations are at a depth of 500 m.

The map of ***return-flow times*** clearly illustrates how the undulating topography of southeastern Sweden may generate both regional and nested local flow systems. Repository locations within the red band have very long ***return-flow times***, whereas repository locations within the blue band have relatively very short ***return-flow times***. Note that zones of red (the highest ***return-flow times***) always should reach up the land surface, as the longest times would occur at locations on the surface in recharge areas. However, because there is a finite amount of dispersion in the numerical model, they do not occur at these locations, and so the results require some interpretation in terms of what an ideal model without dispersion would give.

The primary regional system described in this section has recharge below the topographic peak near the left margin of the top boundary and discharges both to the left boundary (near Vättern Lake) and to the Baltic Sea (top right boundary). Repository locations within the regional system have the longest ***return-flow times*** in the section. The ‘Comparison site’ is located below the primary recharge area for this system. The Hultsfred site spans both low (on the west) and very long ***return-flow times*** (on the east). The coastal sites, Simpevarp and Oskarshamn, have relatively short ***return-flow times***. The eastern portion of the Hultsfred site is below a plateau, which apparently acts as a major recharge area providing high ***return-flow times*** in this area. The coastal sites are in a major discharge area. Although this 2D simulation is a simplification of the 3D model, described later, it qualitatively gives the same results for ***return-flow times*** at the sites considered as does the 3D model for the same set of bedrock properties.

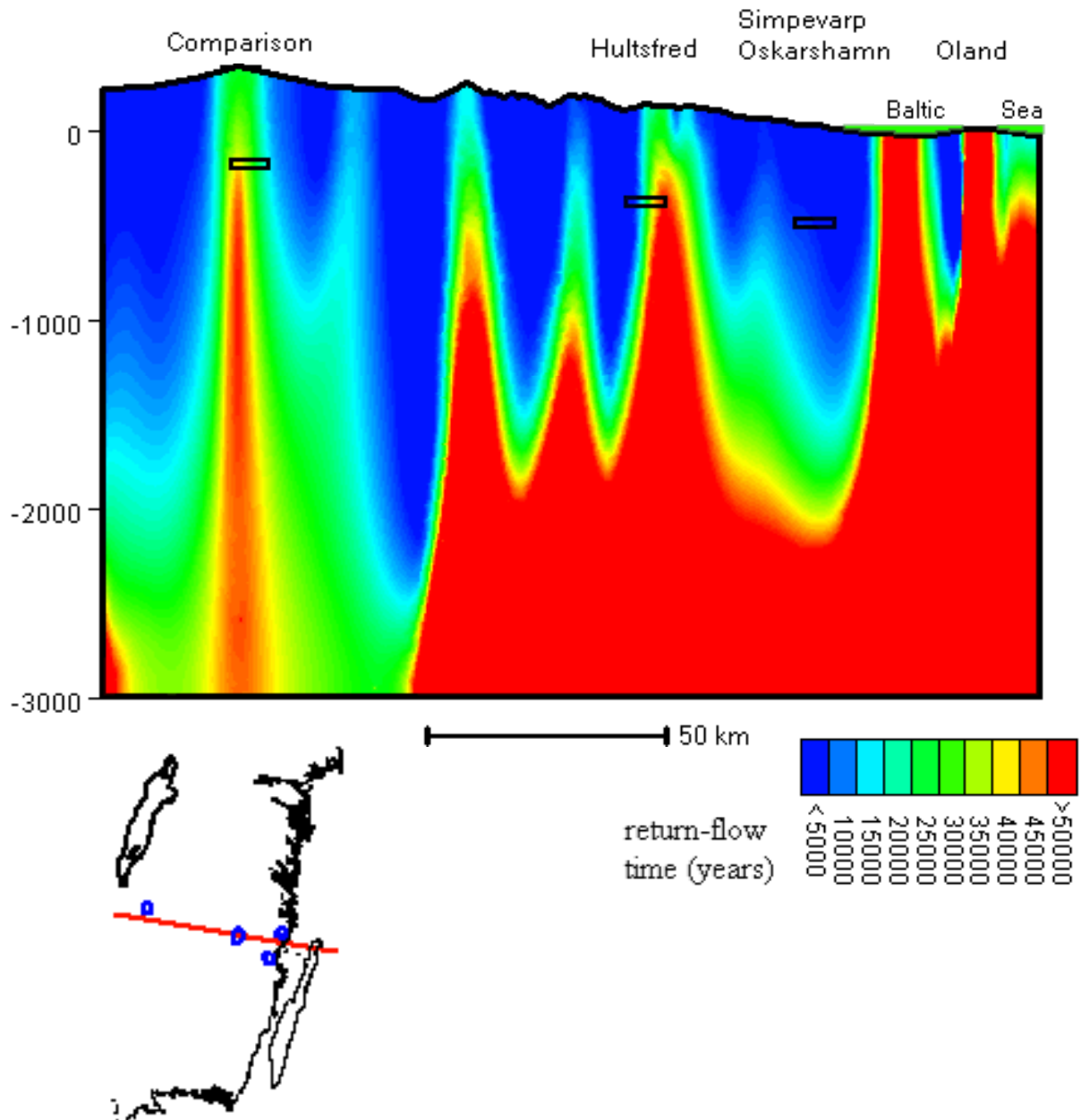


Figure 3. Cross-section through repository sites (black boxes) showing return flow times for illustrative simulation (10:1 Case). Baltic Sea level is shown as green region along top right boundary. Inset shows section location (red line) and repository sites (in blue) and refers to the map in Figure 4. Repository sites – left to right, in cross section – are ‘Comparison’, Hultsfred, and Simpevarp/Oskarshamn. Vertical exaggeration is 40x. Elevation on left edge of cross section is given in m.

Evaluation of Southeastern Sweden Repository Sites

REGIONAL HYDROGEOLOGY

The hydrogeology of southeastern Sweden is considered on a regional scale. This is necessary to allow consideration of regional-scale ground-water flow systems that may exist in the region, which in turn allows identification of recharge areas for the regional flow systems. Flow is considered to be driven by a combination of regional as well as local hydraulic gradients, whereas hydraulic properties can practically only be considered as regional averages because of the local complexity of the conductive fabric of the bedrock.

Study area

This study focuses on the portion of southern Sweden to the east and south of Vättern Lake, with special attention paid to the vicinity of three proposed repository sites: ***Hultsfred east***, ***Simpevarp***, and ***Oskarshamn south*** (SKB, 2001). The study area and model domain are shown in *Figure 4*. The model domain extends beyond the immediate vicinity of these sites to allow the setting of natural hydrologic boundary conditions, to allow the model to naturally generate regional flow systems, and to provide a basis for comparing the proposed sites with alternative sites in southern Sweden.

The land-surface elevation of the study area also is shown in *Figure 4*. Elevation data were obtained from LMV (Lantmäteriverket, 2000) on a grid of 500 m. Normally, in humid regions, the water-table elevation generally follows at some small depth below the land-surface elevation. Therefore, for the purposes of driving ground-water flow in this modeling study, the land-surface elevation is used as an approximation of the water table.

Bedrock hydrogeology

The Fennoscandian shield consists largely of Precambrian granodiorites (> 1700 Ma old) with intruded granitic plutons (~ 1300 Ma old) and occasionally, mafic rocks (Kornfält and Larsson, 1987). Seismic refraction profiles suggest that these rocks extend to a depth of about 20 km in southern Sweden (Lund, 1989). The upper surface of the shield in this region is a Precambrian peneplain.

The crystalline bedrock of the Fennoscandian shield is fractured extensively at scales ranging from microscopic to regional, and these fracture zones may provide regionally connected paths for fluid movement (Voss & Andersson, 1993). However, evidence for large-scale fluid migration

is indirect at best. The present study addresses the uncertainty in regional connectivity of permeable structures by considering the effect of varying degrees of horizontal and vertical fracture connectivity on the regional hydraulic conductivity of the bedrock. The depth dependence of hydraulic conductivity in the shield rock also is highly uncertain. Hydraulic conductivity appears to decrease below a depth of 200 m; measurements to date show no other trend to a depth of about 1 km (Winberg, 1989), and few measurements are present below this depth. A variety of indirect evidence, including information from the Kola super-deep borehole in northwest Russia and widespread seismic low-velocity zones in the Fennoscandian shield and elsewhere, suggest that much of the rock permeability disappears by a depth of 8 to 10 km (Neuzil, 1995). Permeability may decrease due to high temperatures, diagenesis, and prevention of fracture formation or closing of fractures by high lithostatic load.

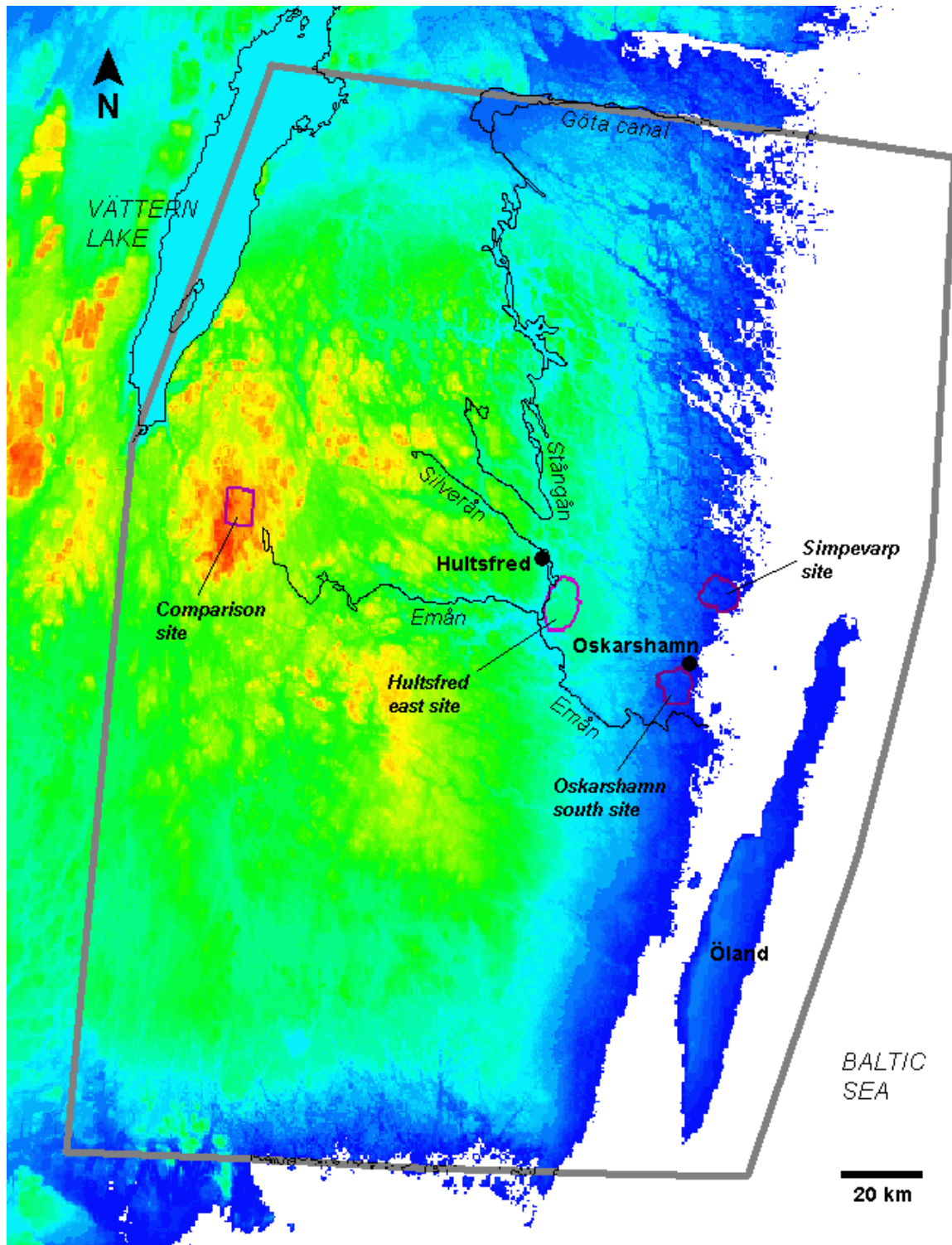


Figure 4. Color topographic map of modeled area in southeastern Sweden showing potential repository sites and Emån/Stångån stream networks. Red is approximately 350 m elevation above sea level (masl). Lake level (not bathymetry) is shown here. The model domain is within the gray boundary.

To the east and southeast of Oskarshamn, crystalline rock is overlain by sedimentary layers extending below the Baltic Sea to central Europe (Kornfält & Larsson, 1987; Ahlbom *and others*, 1990; Grigelis, 1991). Near Sweden, the sedimentary overburden consists of the following sequence: Lower Cambrian sandstone, Middle Cambrian claystone/shale, Upper Cambrian alum shale, Ordovician limestone, Silurian chalk/marl, and Devonian chalk/marl. Relative to the surrounding rock, the sandstone is highly conductive, whereas the shales form an aquitard that extends (at least) 200 km to the east of Oskarshamn (Kornfält & Larsson, 1987).

Flow field and distribution of fluids

The Fennoscandian shield is assumed here to be hydraulically conductive to a depth of about 10 km. Under present-day climatic conditions, subsurface water moves from recharge areas at higher elevations to discharge areas at lower elevations. The coast of Sweden apparently serves as a discharge area for many recharge areas in the regional flow system, whereas at higher elevations inland, smaller-scale recharge-discharge systems occur.

Four types of ground water are generally found in the Fennoscandian shield: recent recharge water of low total dissolved solids (TDS) content, relict glacial meltwater with low TDS, relict seawater at concentrations up to one-third (about 10000 mg/LTDS) of today's ocean water, and shield brine (Glynn & Voss, 1996). The freshwater generally is found at shallow depths at all locations and may be found at greater depths with increasing distance inland from the coast. The regional spatial distribution of the relict glacial meltwater is poorly known; it is found at varying depths at most sites investigated, and likely originates from sub-glacial meltwater of the last glaciation. The relict seawater generally is found in Sweden at locations with surface elevation below about 200 m, and may stem from the period of marine incursion following the last glaciation about 10 ka before present (BP) (Lindewald, 1981, 1985).

Shield brine generally may be found at any location below the fluids described above. Near the coast and other major discharge areas, shield brine is found near the surface, whereas further inland it usually is found at greater depths. Nordstrom and others (1989a) have summarized the various mechanisms that have been proposed for brine formation in crystalline rocks. Both allochthonous (external to the rock) and autochthonous (internal to the rock; arising from rock-water interaction) sources of salinity have been proposed. Mechanisms based on rock-water interaction include silicate mineral hydrolysis (Edmunds and others, 1984) and leakage from saline fluid inclusions (Nordstrom and others, 1989b).

Because of the large density contrast between freshwater and shield brine, circulation of shield brine likely is driven by both fluid density differences and topographic gradients, whereas flow of the more dilute fluids, including relict seawater, is driven primarily by topographic gradients (Voss and Andersson, 1993).

The complex pattern of fracture zones and permeable structures in the shield suggests that the three-dimensional spatial distribution of fluid types may be complex. Therefore, these distributions and the flow field are difficult to interpret in detail for any particular area, even on the basis of intensive hydrogeologic field programs. Any detailed description of the spatial distributions of permeable structures and fluids is certain to be incomplete, at best, and the evolution of these distributions through time is subject to even more uncertainty. Most often, only broad generalizations of the kind made in this study can be advanced with any confidence.

DESCRIPTION OF THE 3D MODEL

Extent and discretization of the 3D model

The model covers a roughly rectangular area that includes the portion of Sweden to the east and south of Vättern Lake. The model domain extends approximately 260 km north to south and 210 km east to west, covering an area of approximately 49,000 km² (*Figure 4*). Vertically, the model extends from the land surface (with a variable elevation given by the digital elevation grid) to a depth of 10 km below sea level. The model U.S. Geological Survey's SUTRA code was used in three spatial dimensions (Voss and Provost, 2001, personal communication). Input data for the 3D model were generated using a 3D version (Winston and Voss, 2001, personal communication) of SutraGUI (Voss and others, 1997), a graphical interface for the SUTRA code.

The finite-element mesh of the region in plan view is shown in *Figure 5*. An oblique view of the three-dimensional mesh exaggerated ten times in the vertical direction is shown in *Figure 6*. The finest discretization is in the vicinity of the Hultsfred site, near the land surface, where the elements measure approximately 1,500 m on a side horizontally and 250 m vertically. The coarsest discretization is in the southwestern portion of the model, near the bottom, where the elements measure roughly 10,000 m on a side horizontally and 1,000 m vertically. The distribution of elements was designed to provide finer discretization in the shallow subsurface in the vicinity of the sites and in the network of stream valleys that runs among the sites (including portions of the streams, Strångån, Silverån, and Emån, see *Figure 4*), which was expected to strongly affect the flow field. The mesh has 15 elements vertically, 58 east-west, and 64 north-south, totaling 55680 elements and 61360 nodes. This mesh must be considered as a rather coarse discretization of the region; however, present limits on convenient computing resources preclude the use of finer meshes.

The upper surface of the model is the water table, equated here to the land-surface elevation as derived from LMV (Lantmäteriverket, 2000) topographic data on a grid of 500 m. The original data were first converted (using a computer program written for this purpose) to a gridded data format and then imported into a data layer of the SutraGUI graphical user interface. To reduce the amount of storage required and to speed up calculations, the data were subsequently transferred to a second, "meshed" data layer that stores elevations only at the nodes of the finite-element mesh.

The LMV data give lake water levels, not bathymetries. Equating the surface of the model with lake water levels is considered unlikely to cause substantial error, given the relatively small area and shallow depth of the lakes in the region compared with the model domain. The exception is Vättern Lake, which is relatively large and deep. Beneath Vättern Lake and the Baltic Sea, the surface of the model is based on bathymetric data (Tirén, S., 2000, personal communication), which were represented as contours in the graphical user interface and transferred to the meshed data layer along with the land surface elevations.

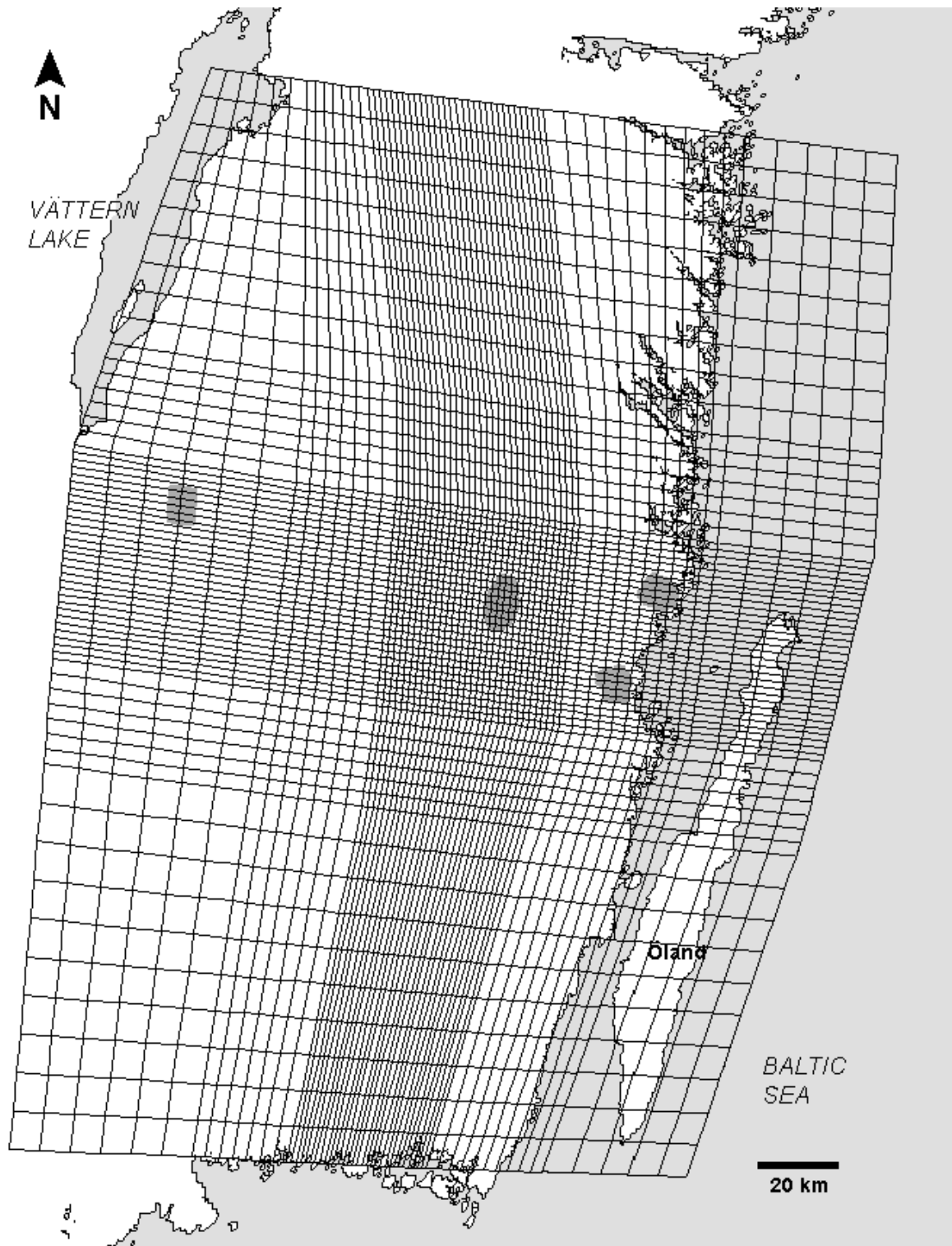


Figure 5. Map view of three-dimensional finite-element mesh, showing potential repository sites (gray regions).

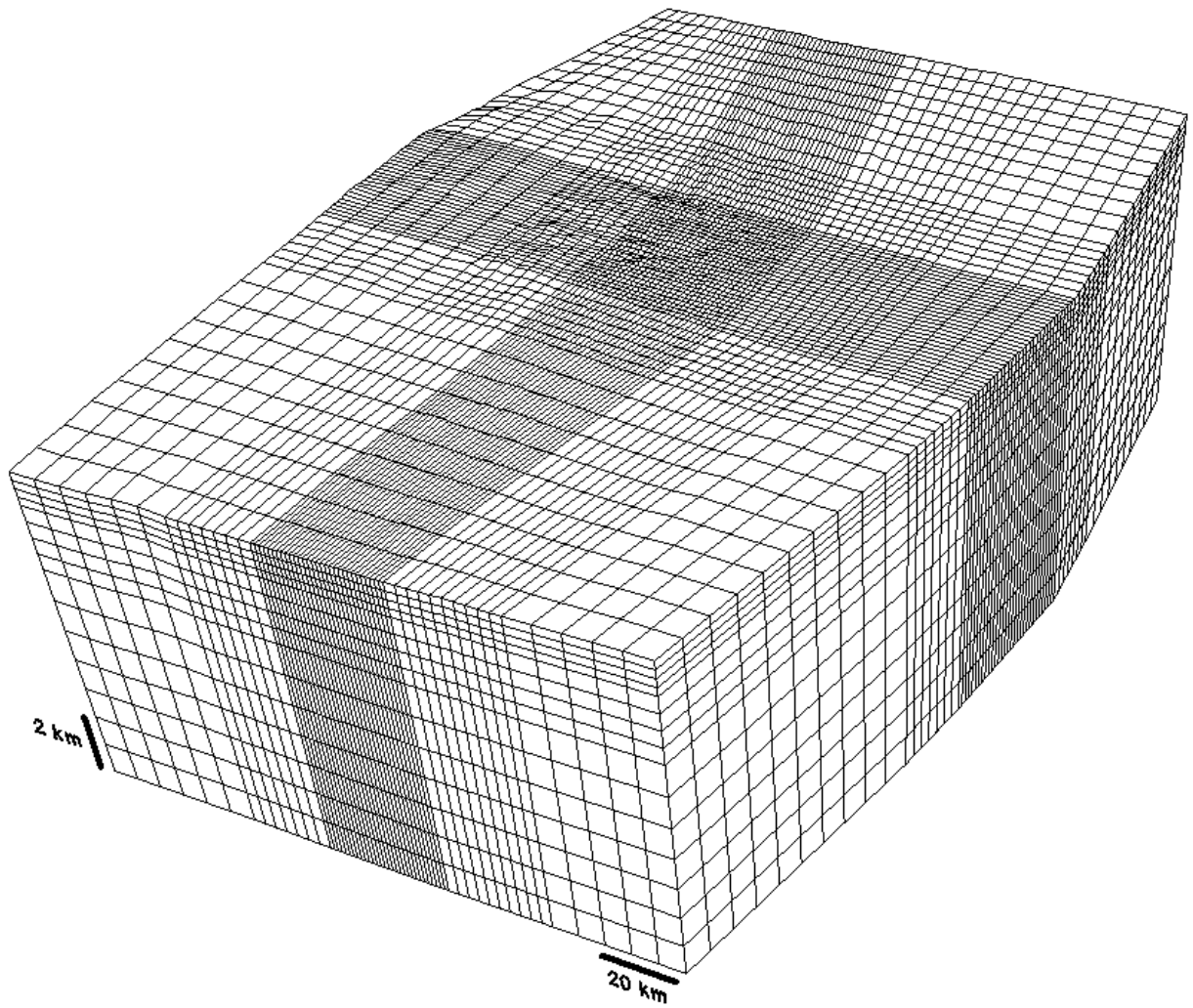


Figure 6. Oblique view (from southeast towards northwest) of three-dimensional finite-element mesh. (Vertical exaggeration = 10x)

Representation of the bedrock fabric

Crystalline bedrock is treated here as a continuum with an effective hydraulic conductivity that accounts for the presence of the regional network of fracture zones. The conductivity may be anisotropic (different values for different flow directions) or isotropic (same values for all flow directions). Individual fracture zones are not resolved at the regional scale of this model; indeed, a three-dimensional map of fracture zones is not available for the region and would be impossible to create with any certainty. Hydraulic conductivity and anisotropy of hydraulic conductivity must be considered on a large scale for purposes of modeling.

The sedimentary units along the southeast coast are not included in the present model, and the region where these layers are present is assigned properties of granodiorites equivalent to the rest of the model. It may be expected that the presence of the sediments as a confining unit would tend to force discharge from the terrestrial flow system closer to the east coast of Sweden than in the case where they are neglected, shortening travel times and flow paths for near-coastal repository locations. Thus, neglecting the confining units in this analysis is an optimistic approximation for near-coastal repository locations because it lengthens travel times and flow paths for these locations.

Decreases in conductivity with depth could be represented in the model by a layered conductivity profile; however, these are not considered, and all simulations assume constant conductivity with depth. The subsurface fluid density increases with depth in the simulations, and fluid flows are substantially lower in the denser fluids. Thus, the presence of high density at depth in the model gives an effect on the shallower flow field and on the decrease in fluid flux with depth similar to that of a decreasing conductivity with depth. Decreasing the conductivity at depth would most likely have the greatest effect on long, deep flow paths. Some of these flow paths could be displaced into shallower, more conductive layers, thereby decreasing their return-flow times, whereas the return-flow times for flow paths that remained in deeper, less conductive layers would likely increase. Given the considerable uncertainty in the variation of conductivity with depth and in the spatial distribution of fluid density, it was decided not to include depth-dependent conductivity in the present analysis.

Hydraulic permeability and porosity in the crystalline basement are based on the ranges of values given by Voss & Andersson (1991, 1993) for various assumptions based on Sweden's bedrock, about fracture zone spacing and direction (e.g. spacing of horizontal zones on the order of hundreds of meters and spacing of vertical zones on the order of thousands of meters). When determining an effective hydraulic conductivity for model simulation, conductive fracture zones are assumed to have thickness of about 50 m and conductivity of about 10^{-7} m/s, and the rest of the bedrock is assumed to have conductivity of about 10^{-10} m/s. Overall porosity is set uniformly at 10^{-3} . It is assumed here that fracture zone intersections do not enhance conductivity and do not act as conduits.

Simple calculations for equivalent porous media representations of fractured rock (using in-series and in-parallel models of fracture zones and the host rock) show that the bedrock, on a regional scale, may not be extremely anisotropic (*Table 1*). If the rock contains only a single set of parallel conductive fracture zones (with the conductivities described above) then the maximum possible anisotropy in hydraulic conductivity is less than 100:1 (*maximum permeability : minimum permeability*) for spacing of about 1 km and about 20:1 for 5 km spacing, with the higher conductivity value in the directions parallel to the fracture zones. For a case with two nearly perpendicular sets of equally spaced parallel fracture zones (each with the same properties and spacing), the maximum possible anisotropy in hydraulic conductivity is only 2:1, with the higher value oriented along the direction of intersections of the two sets of fracture zones. For three equally spaced mutually perpendicular sets of parallel fracture zones, the bedrock conductivity tends to be approximately isotropic (e.g., 1:1 or 10:10). If spacing of one set of the three were different from the others, then anisotropy could be higher; for example, 10:1 or 20:1 for typical spacings.

For the bedrock within the model area of southeastern Sweden, it may be speculated that it is unlikely that there is only one set of conductive fracture zones. If there are two conductive mutually perpendicular fracture zone sets throughout, then the regional scale anisotropy is on the order of 2:1. If the two sets are aligned horizontally and vertically, then horizontal conductivity along the direction of the zone intersections is about two times higher than in the other directions. If the two sets are both aligned vertically, then the vertical conductivity is about two times higher than in the horizontal directions. If there are three mutually perpendicular fracture zone sets throughout the bedrock, then anisotropy may range from isotropic for equal spacing and properties (e.g., 1:1 or 10:10) to about 10:1 or 20:1 for unequal spacing or properties, probably with the horizontal conductivity higher than the vertical conductivity. Thus, the full range of anisotropy in effective hydraulic conductivity that is possible in the bedrock is between about 2 times higher vertically than horizontally to 20 times greater horizontally than vertically. This range includes the possibility of isotropic bedrock.

For the numerical modeling, four cases are considered with respect to distribution of fracture zones in the bedrock. These cases are chosen to represent the full range of possible average anisotropies of the bedrock. In this study, areal anisotropy is not considered, and the bedrock is allowed only horizontal-vertical anisotropy. In the case with only vertical zones, local topographic gradients would have their greatest effect on the system, resulting in deeply penetrating local flow cells. This case would result in the shortest flow paths from a repository and the poorest performance of an upstream repository. To exaggerate this pessimistic case for recharge-area repositories, although the greatest possible vertical anisotropy is only 1:2, a case is considered here with anisotropy of 1:10, where the vertical conductivity is 10 times higher than horizontal conductivities. The situation that would result in the largest regional flow systems is the one with the highest horizontal anisotropy, or 20:1, where vertical conductivity is less than horizontal conductivity. To reduce the advantage of this situation for recharge area repositories, only a case of 10:1 is considered.

The four cases selected to represent the possible range regional anisotropy in hydraulic conductivity of the bedrock are given below.

10:1

Average horizontal hydraulic conductivity of the bedrock is 1.0×10^{-8} m/s and average vertical conductivity is *10 times less*, 1.0×10^{-9} m/s. These values derive from an equivalent porous medium representation of bedrock with two perpendicular sets of sub-vertical fracture zones and a set of sub-horizontal fracture zones (Voss & Andersson, 1991, 1993). Zones are about 50 m wide, with a vertical spacing of about 1 km and a horizontal spacing of 5 km. Similarly, these average values represent a family of bedrock configurations that includes, for example, one with 25 m wide zones with vertical spacing of 0.5 km and horizontal spacing of 2.5 km. The rock has anisotropic conductivity with horizontal conductivities 10 times greater than vertical conductivity, or '10:1' anisotropy.

10:10

This case represents bedrock that consists of three mutually perpendicular sets of parallel fracture zones (e.g., one sub-horizontal set and two vertical sets) with approximately the same spacing in each direction. There is about one 5-m wide conductive zone per km in all directions. The resulting average bedrock conductivity is approximately isotropic with conductivities of 1.0×10^{-8} m/s.

1:1

The 1:1 case represents the same situation as the 10:10 case, but where overall conductivities are 10 times lower (1.0×10^{-9} m/s). This case is equivalent to fewer conductive zones per km and/or lower zone widths.

1:10

This case represents the bedrock where there is an extremely high vertical conductivity in comparison with the horizontal conductivity. The case derives from an equivalent porous medium representation with two perpendicular sets of conductive sub-vertical fracture zones and without well-connected conductive sub-horizontal fracture zones. Average vertical hydraulic conductivity of the bedrock is 1.0×10^{-8} m/s and average horizontal conductivity is *10 times less*, 1.0×10^{-9} m/s. The 1:10 case would represent an extreme vertical anisotropy for bedrock with two or more intersecting sub-parallel fracture sets as described above. However, it is considered here to establish return-flow time results for a bounding situation in the bedrock where local topographic gradients potentially are more important than semi-regional or regional gradients in establishing flow, and which may produce the shortest flow paths and travel times possible from a repository, given the discretization presently used in the finite element mesh.

<i>Number of zone sets</i>	<i>Direction of zone sets</i>	Relative Hydraulic Conductivity			<i>Spacing of zones</i>
		Max (direction)	Mid (direction)	Min (direction)	
<i>1</i>	<i>H or V</i>	100	100	1	<i>1 km</i>
		20	20	1	<i>5 km</i>
<i>2</i>	<i>V₁ and V₂</i>	2 (vert)	1 (hor1)	1 (hor2)	<i>V₁ (Any)</i> <i>V₂ (same as V₁)</i>
		2 (hor1)	1 (hor2)	1 (vert)	<i>H (Any)</i> <i>V (same as H)</i>
<i>3</i>	<i>H and V₁ and V₂</i>	1	1	1	<i>H (Any)</i> <i>V₁ (same as H)</i> <i>V₂ (same as H)</i>
		10 (hor1)	10 (hor2)	1 (vert)	<i>H (1 km)</i> <i>V₁ (5 km)</i> <i>V₂ (5 km)</i>
		20 (hor1)	20 (hor2)	1 (vert)	<i>H (.5 km)</i> <i>V₁ (5 km)</i> <i>V₂ (5 km)</i>

Table 1. Regional effective anisotropy of bedrock containing sets of conductive fracture zones. Principal hydraulic conductivity values in maximum (**Max**), middle (**Mid**) and minimum (**Min**) directions of anisotropic hydraulic conductivity are given relative to the minimum value. Depending on the configuration of zones, these values may be oriented in the vertical direction (**vert**), or in one of the mutually perpendicular horizontal directions (**hor1** or **hor2**). Each set is assumed to occur as sub-parallel zones oriented either horizontally (*H*) or vertically (*V*, *V₁* and *V₂*). Zone widths are assumed to be much less than the spacing between zones. Two or more sets of zones occurring simultaneously are assumed to be mutually perpendicular. Each zone is 50-m wide with hydraulic conductivity of 2×10^{-7} (m/s) and the remaining bedrock has hydraulic conductivity of 1×10^{-10} (m/s). Spacing of zones in each set is given in the table. Spacings are equal when referring to more than one set. These anisotropies proportionally also represent other zone spacings, widths, and hydraulic conductivity.

Modeling of physical processes and boundary conditions

The U.S. Geological Survey computer code SUTRA (Voss, 1984, two-dimensional code) was upgraded for three-dimensional simulation (Voss and Provost, 2001, personal communication) and used to model variable-density fluid flow in southeastern Sweden. Variable fluid density is required for model simulation because of the presence of shield brines at depth in the bedrock with significantly higher density than the freshwater found in shallower regions. For each bedrock permeability case considered, a steady-state flow and solute concentration was simulated to represent present-day conditions. Repository siting then was evaluated on the basis of this flow field. It is not known whether the present ground-water system has reached a steady state following the substantial glacial-climatic stresses experienced over the past 20 Ka. However, Provost and others (1998) have shown that in the amount of time elapsed in the present interglacial, it is possible to nearly reach a steady state.

The model considers the formation of shield brine as a process that depends on the difference between concentration of dissolved solids in the ground water and the potential dissolved solids concentration contributed by rock weathering, leaching of saline fluid inclusions, or other processes (Nordstrom and others, 1989a). The rate of brine formation is represented (Provost and others, 1998) by a first-order expression, $\text{rate} = k_{\text{mt}}(C^{\text{max}} - C)$, where C is the concentration of total dissolved solids in the ground water, C^{max} is the maximum allowable concentration of dissolved solids, and k_{mt} is a rate constant (or mass transfer coefficient) for rock-water mass transfer. In the case of rock weathering, C^{max} could represent the saturation concentration of dissolved solids (treated as a single species), whereas in the case of leaching of inclusions, it could represent the concentration of dissolved solids in the inclusions.

The simple expression, defined above, for the rate of brine formation captures two essential features of the rock-water interaction: the rate at which solids dissolve in the pore fluid decreases as their concentration in the fluid increases and dissolution ceases when the concentration reaches the maximum allowable value. Despite its simplicity, this expression can be adjusted to approximate the general present-day trend in the concentration profile of dissolved solids observed in the field, and its incorporation into the model allows the density distribution to evolve along with the pressure field in a "natural" and consistent way. The expression does not depend on what particular geochemical process actually generates the shield brines, as it accommodates a number of possible origins. The value of the mass transfer coefficient, k_{mt} , was not known *a priori*. Rather, its value was chosen by trial-and-error to give realistic distributions of shield brine in the model for each bedrock case.

An alternative approach to simulating variable-fluid-density flow would be to impose a fixed density distribution based on field measurements throughout the domain. However, field measurements are sparse. Furthermore, the imposition of such an "artificial" concentration profile causes spurious flows because the flow field is very sensitive to the interplay of pressure and buoyancy forces. The brine formation mechanism employed in this work avoids such spurious flows by providing mutually consistent pressure and concentration fields.

In each case of bedrock hydraulic conductivity considered, k_{mt} was chosen such that a concentration of $0.1(C^{max})$ was located at a maximum depth of 3 to 5 km beneath the southern Swedish highlands. This approach provided an approximate correspondence with the 2D results of Provost and others (1998), which in turn were approximately calibrated to concentrations measured in two boreholes, one within the region of the present model. Thus, for each case, the model was adjusted such that the modeled concentration field approximately matched the available salt-distribution data.

A no-flux condition is imposed at the bottom boundary of the model to represent the depth at which the bedrock permeability is so small (because of lithostatic loading and high temperatures) that it can be considered zero for this model. The western boundary is assumed to be approximately a ground-water divide, and a no-flux condition is imposed there. The northern boundary is assumed to be approximately along a streamline, and a no-flux condition is imposed there. The southern boundary is a no-flux boundary that approximately follows the coastline, under the assumption that most southward ground-water flow will discharge upward upon reaching the Baltic Sea. Likewise, the offshore boundary to the east is assumed to be a no-flux boundary. Both the southern and eastern boundaries are assumed to be sufficiently far removed from the area of interest that the model-simulated terrestrial flows would not be affected by the choice of hydrostatic pressure or no-flux as boundary conditions at these locations.

On land, the top boundary of the model is set to a specified pressure of zero (i.e., atmospheric pressure), and any inflow that occurs along the top boundary is assigned a salt concentration of zero (freshwater) for the steady-state flow-field simulation. The elevation of the top surface of the model is intended to represent the water table on the land portion, and is set to the value given by the elevation grid. Beneath Vättern Lake and the Baltic Sea, the top of the model follows the bathymetry, and pressure is set as the hydrostatic value according to the depth and density of each type of water. Vättern Lake elevation is set to 89 m (Tirén, S., 2000, personal communication). The concentration assigned for lake water is zero; the concentration assigned for Baltic Sea water is 0.005 kg-solute/kg-fluid (about 1/7 standard seawater).

Dispersion coefficients for transport of salt are set to relatively low values, considering the coarse discretization used in the model. The values are chosen arbitrarily, as no data are available for transport in the Fennoscandian shield at the scales of transport considered in this model domain. However, the values are not unreasonable for the scale of solute transport in the system under consideration.

Model input data

Values for model parameters for each of the bedrock cases are given in *Table 2*.

Parameter	Case			
	10:1	10:10	1:1	1:10
rate of change of fluid density with solute mass fraction, (kg/m ³)/(kg-solute/kg-fluid)	875.5			
fluid viscosity, kg/(m s)	0.001			
solid matrix compressibility, [kg/(m s ²)] ⁻¹	1.0x10 ⁻¹⁰			
rate constant for rock-water mass transfer, s ⁻¹	2.0x10 ⁻¹⁴	3.0x10 ⁻¹⁴	4.5x10 ⁻¹⁵	6.5x10 ⁻¹⁵
maximum allowable solute concentration, kg-solute/kg-fluid	0.3			
acceleration of gravity, m/s ²	9.81			
porosity	0.001			
horizontal permeability, m ²	1.02x10 ⁻¹⁵	1.02x10 ⁻¹⁵	1.02x10 ⁻¹⁶	1.02x10 ⁻¹⁶
vertical permeability, m ²	1.02x10 ⁻¹⁶	1.02x10 ⁻¹⁵	1.02x10 ⁻¹⁶	1.02x10 ⁻¹⁵
longitudinal dispersivity for flow in the horizontal directions, m	1500.			
longitudinal dispersivity for flow in the vertical direction, m	250.			
transverse dispersivity for flow in the horizontal directions, m	150.			
transverse dispersivity for flow in the vertical direction, m	25.			

Table 2. Input parameters for four models of variable-density ground-water flow and brine transport in southeastern Sweden, corresponding to four cases representing properties of the bedrock. Entries not shown are the same as for the 10:1 Case.

RESULTS OF 3D MODELING

The flow field is evaluated and path lengths and travel times from each of four repository locations in southeastern Sweden are determined for the four cases of the bedrock permeability. As described above in the section, “**Representation of the bedrock fabric**”, these four cases are intended to represent the range of effective large-scale bedrock properties that is possible. The three sites under consideration by SKB as potential high-level nuclear waste repositories are *Simpevarp*, *Oskarshamn-south*, and *Hultsfred-east* (see *Figure 4*). Additionally, an alternative site with one of the longest *travel times* and *path lengths* within all of southeastern Sweden for all bedrock permeability cases considered (as determined from all simulations carried out) also is presented for purposes of *comparison* (see *Figure 4*). All repository sites are located at a depth of 500 m below the top surface of the model.

Steady-state flow fields and salt distributions

A 3D view of the complete model domain shows the steady-state salt distribution for the bedrock 10:1 Case (*horizontal permeability: vertical permeability = 10:1*) (*Figure 7*). Shading indicates the relief on the top surface of the model. Dark blue indicates the fresher water and generally covers the land surface; recharge to Öland is visible in the dark blue stripe offshore.

The internal steady-state salt distribution for each bedrock case along a vertical section that runs approximately east to west through the *Simpevarp* site is shown in *Figure 8*. Because the permeability distribution was different for each case, the mass transfer coefficient, k_{mt} , was adjusted, in each case, by trial-and-error in a sequence of simulations until a near-steady salt distribution was achieved that matches available field data. These field data were used for a similar purpose by Provost and others (1998). (Simulations were performed using the ORTHOMIN iterative solver for pressure and concentration.)

A close view of the vertical cross section for the 10:1 Case in *Figure 9* highlights the variable-density flow field. This section is similar to that shown in *Figure 3*, and the *return flow times* shown in *Figure 3* may be approximately compared with the velocities shown in the upper part of *Figure 9* (10:1 Case). (A map of return flow times based on the 3D model along the section in *Figure 9* could be generated based on the same procedure used to generate the return flow time maps at 500 m depth. These maps are shown and discussed later in this report. It was deemed unnecessary to show these for the purpose of the present illustration.) Note that the areas of downflow near the surface in *Figure 9* (10:1 Case) are in regions where the *return flow times* are very high near the surface in *Figure 3*. These are the areas of recharge to the deeper ground-water-flow systems. Recharge is at relatively higher elevations, and discharge is at relatively lower elevations of the top surface. The flow field is complex as a result of rotational flow due to variable density and because of the undulating surface topography. A distinctive feature is the zone of discharging fluids near the coast. Note also that flow velocities at depth are very low; these low velocities are a result of the dense fluids at depth, which retard the deep penetration of the fresher, more buoyant water that is recharged at the surface.

Ideally, a repository could be placed in a recharge area for the deep flow systems to maximize *return flow times*. For the 10:1 Case, the *Comparison* site and eastern portion of the *Hultsfred-east* site are in broad-deep regions of downflow, whereas the coastal *Simpevarp* and *Oskarshamn-south* sites clearly are located within the coastal discharge area. For the 1:10 Case (lower part of *Figure 9*) the vertical components of flow are much greater (relative to horizontal) than in the former case. In this case, however, the *Comparison* site is still in a broad, deep region of downflow. Also, much of the *Hultsfred-east* site is still in a broad, deep region of downflow, though there may be a small upflow near the easternmost margin of the site. The *Simpevarp* and *Oskarshamn-south* sites are still clearly located within the coastal discharge area, though there are greater downward components of flow in parts of the sites than in the former case.

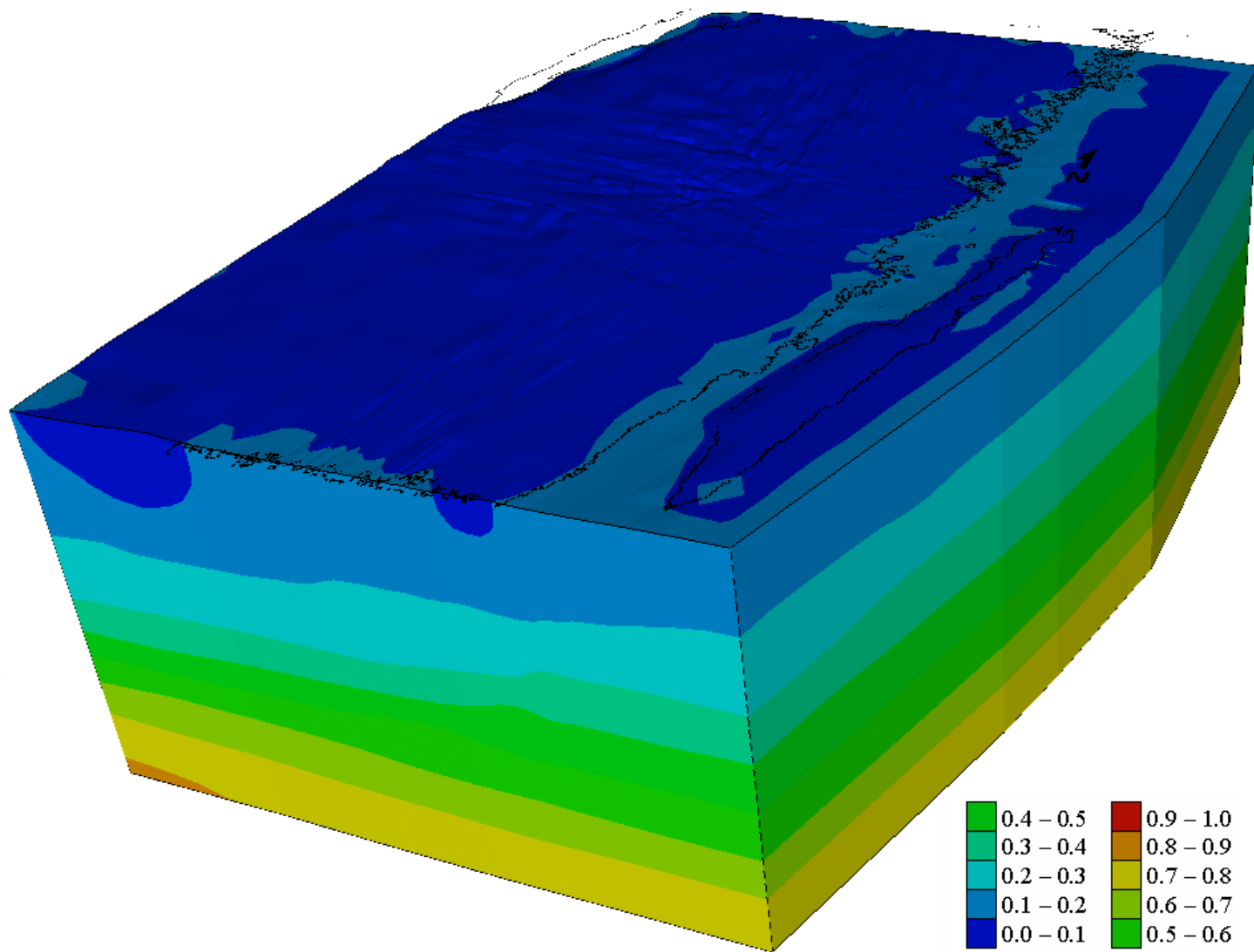


Figure 7. Salt distribution for permeability 10:1 Case (*horizontal permeability : vertical permeability = 10:1*). View from southeast of entire model. Concentrations shown are fractions of maximum salt concentration. Vertical exaggeration = 10x.

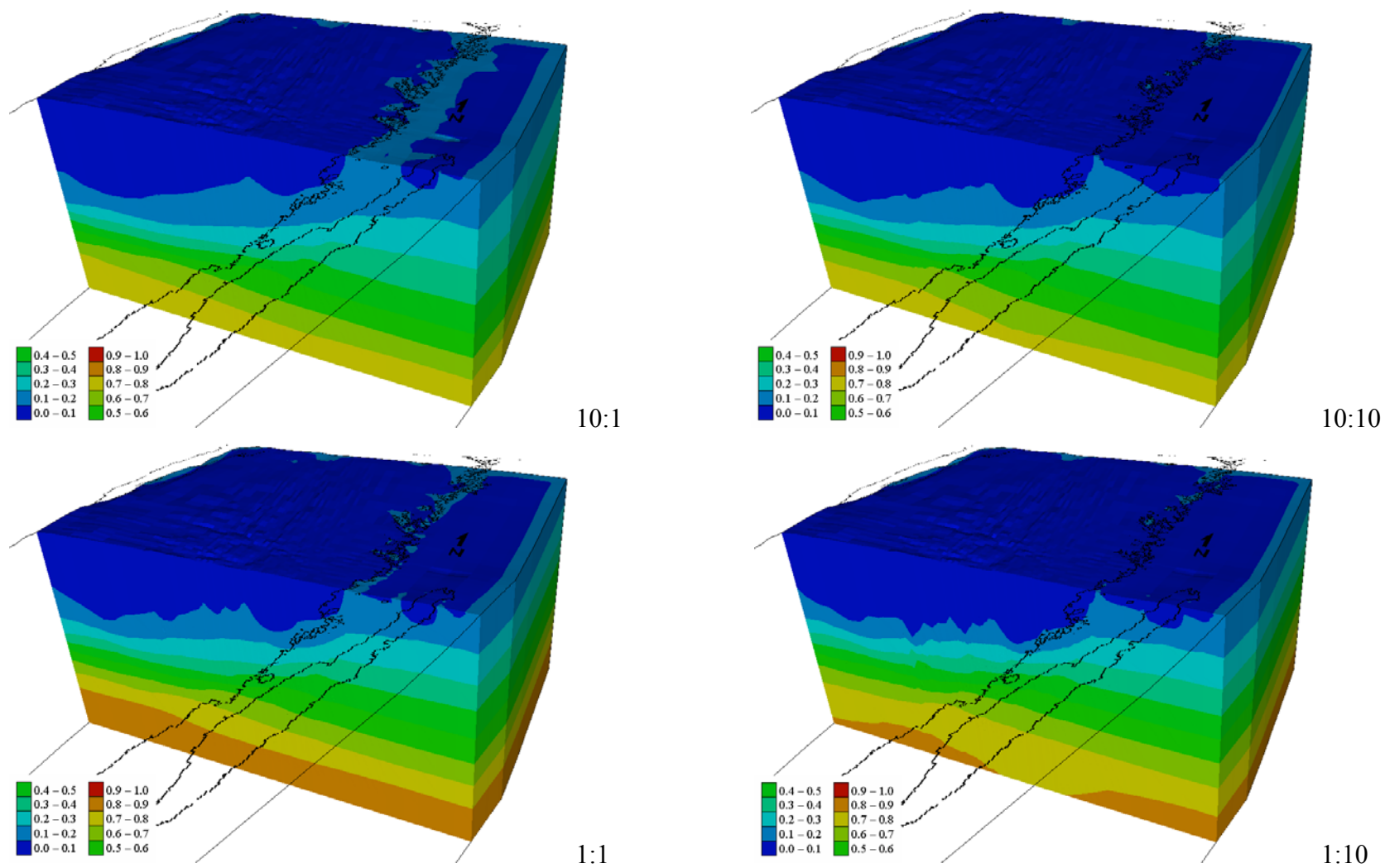


Figure 8. Salt distributions for four bedrock cases (*horizontal permeability* : *vertical permeability*). View from southeast. East-west section approximately through Simpevarp. Vertical exaggeration = 10x. Concentrations shown are fractions of maximum salt concentration.

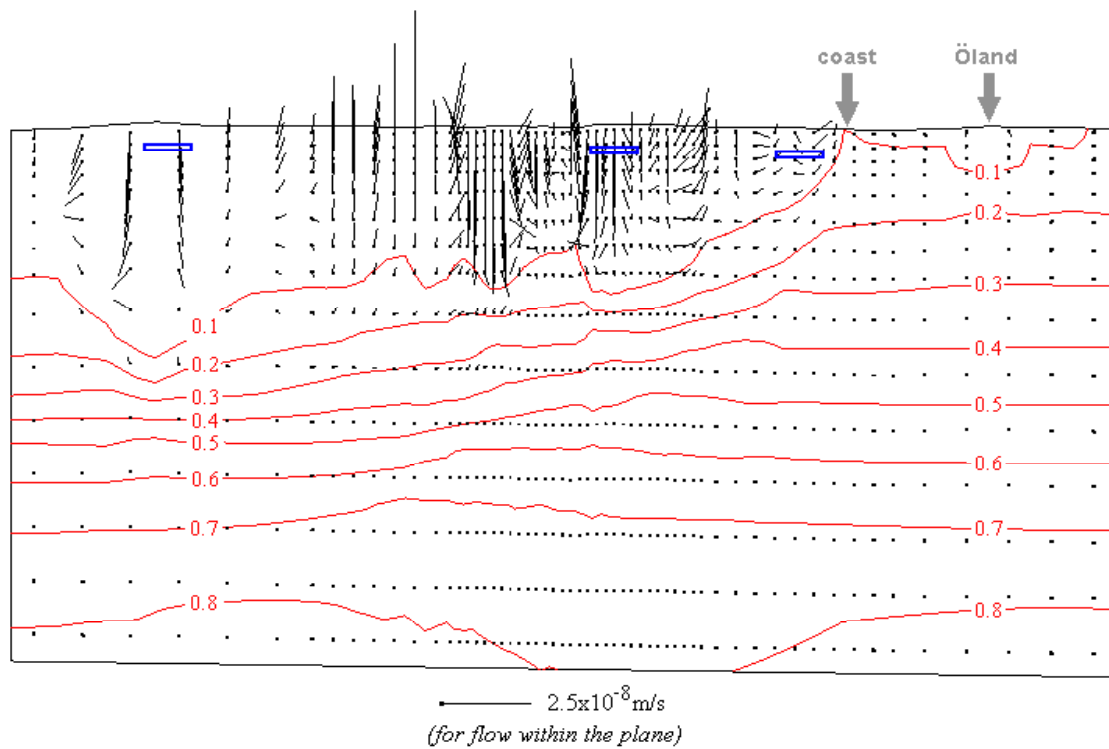
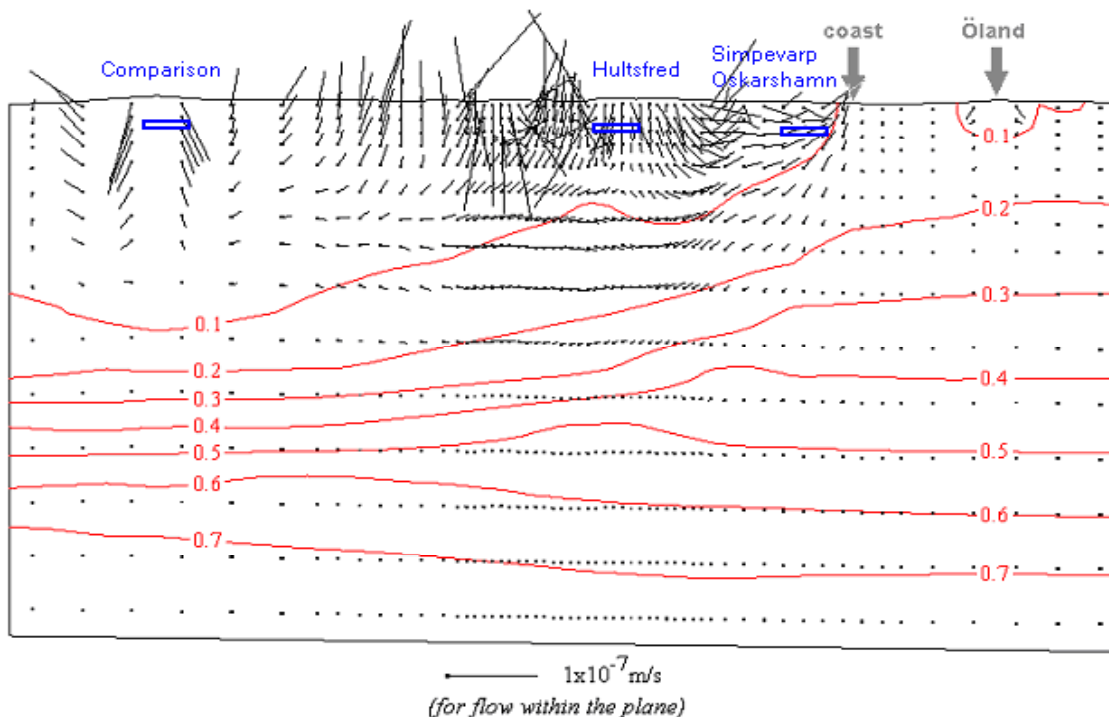


Figure 9. Salt concentration and flow field for bedrock Cases (10:1) and (1:10). Cross-section viewed from the south. (Similar, though not identical, section as in oblique view of *Figure 8*.) Vertical exaggeration = 10x, total depth of section is about 10 km. Concentrations shown are fractions of maximum salt concentration. Velocity vectors are shown projected onto the plane of cross-section. Flow direction is in the direction of the line extending from the dot that represents the base of the vector and vector length is proportional to velocity. Approximate locations of potential repository sites are shown in blue.

Flow paths

Flow paths for each repository site are shown as colored 3D flow tubes for the two bounding bedrock cases, 10:1 and 1:10, in *Figure 10* and in *Figure 11*. These two cases should provide the longest (for 10:1) and shortest (for 1:10) possible paths among the bedrock models considered here. The length of the *flow path* from each repository in each of these two cases may be estimated from the model result. These lengths are summarized in *Table 3*.

Each simulation results in a flow tube traced in 3D from the repository location to the point(s) of exit from the model domain. A flow tube may exit at one location or it may split and exit at a number of locations. The flow tube represents a plume of conservative solute emanating from the entire repository site. Ideally, flow tubes in 3D should be traced in a model with no dispersion. As this is not possible in these model simulations, flow tubes must be traced using solute transport with dispersion and by tracking concentration.

The flow tube approximately traces out the subsurface region into which any escaping radionuclides from a repository would potentially spread. In these simulations, dispersion was kept near the minimum required to maintain numerical stability. Higher dispersion in the model tends to widen the flow tubes, possibly causing solute to enter adjoining flow paths that do not pass through the repository, and thus would tend to exaggerate the flow-tube volumes. Flow tubes are visualized by selecting the lowest value of concentration whose isosurface exhibits a minimum of branching and also connects the repository with discharge points. Sometimes, it is not possible to track a low concentration from the site to its ultimate discharge point because the flow path is obscured by the dispersion required to maintain numerical stability in the coarse mesh. In this situation, the *path length* in *Table 3* is indicated as being greater than the *path length* that could be tracked. The volumes of the flow tubes, as visualized in the present model, must be considered as approximate indicators of the actual *path volumes*, and should be used for relative comparison among the sites; this is both because dispersion is not zero and because an arbitrary concentration level is selected for tube display.

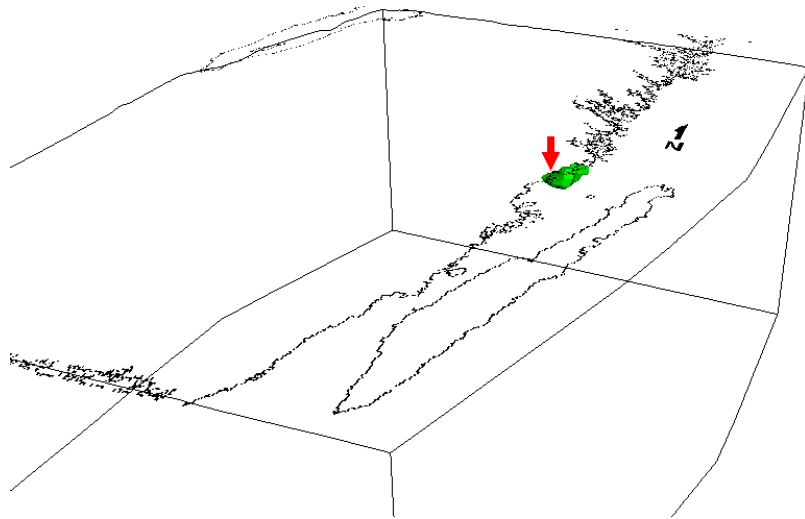
Flow paths are simulated by selecting a set of nodes in the mesh at a depth of 500 m in the designated repository area at which concentration is specified as an arbitrary non-zero value. The concentration of recharge from the top surface is set to zero. A single transport solution for a very long time step (e.g., 10^{13} years) is obtained (with the GMRES solver) to approximate steady transport conditions given the flow field from the steady-state variable density simulation for each case.

To facilitate tracking of advective tracer transport, an effort was made to use the minimum amount of dispersion possible while maintaining numerical stability. For the 10:1 case, the longitudinal dispersivities are set to $\frac{1}{4}$ of the representative mesh spacing in the direction of flow: for horizontal flow, they range from 450 m in the central portion of the mesh to 2500 m in the outer portion; for vertical flow, they range from 62.5 m near the top surface to 500 m near the bottom of the model. For the 1:10 case, the longitudinal dispersivities are set 3 times as high as for the 10:1 case. In both cases, the transverse dispersivities are set to 10 m throughout the model.

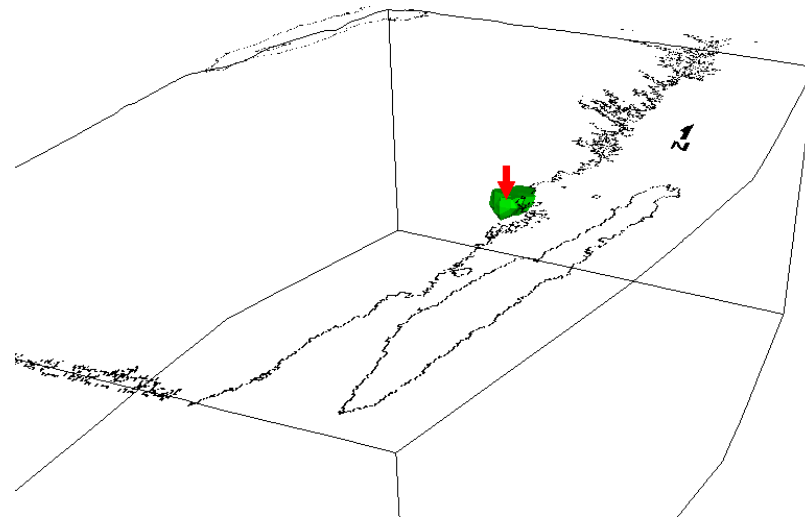
It may be seen that, in the 10:1 Case, the *flow paths* from the ***Simpevarp*** and ***Oskarshamn-south*** repository sites near the coast are very short, nearly vertical and tending upwards towards the coast (*Figure 10*). *Path lengths* from these sites are about 3 km or less. (Because of the large size of the finite elements in the area, *path lengths* shorter than 3 km, which may be possible in these locations, cannot be resolved.) These *path lengths* are about an order of magnitude shorter than the longest paths from ***Hultsfred-east***, where flow occurs from west to east (left to right in *Figure 10*) over a distance of about 25 km before discharging at an inland point and a distance of about 50 km before discharging at the coast. The ***comparison*** site, situated in a highland area in the western portion of the region, has the longest *flow path* that discharges at the east coast. The *path length* is about 130 km. Another flow tube from the site discharges at Vättern Lake, a distance of about 60 km. At both ***Hultsfred-east*** and the ***comparison*** site, the path begins in a direction directly downward from the repository location for a few km before becoming horizontal.

In the 1:10 Case, the *flow paths* from the ***Simpevarp*** and ***Oskarshamn-south*** repositories near the coast are again very short, about 3 km or less (*Figure 11*). At the ***Hultsfred-east*** site, the flow tube leaves the repository directly downwards until at a few kilometers depth, it turns upwards towards the coast and then becomes difficult to track further in the model. The *path length* is at least 30 km, as far as the tube can be tracked in the simulation. There also are some local upflows from some locations within the site with very short *path lengths*. At the ***comparison*** site, the flow tube leaves the repository directly downwards until at several kilometers depth, it splits into two parts. One tube travels to the east, but concentrations in the simulation are too low to track the plume to its discharge point, possibly at the coast (*path length* at least 45 km long). The other tube rises towards a discharge point south of Vättern Lake along the model boundary (*path length* about 35 km long). At the margins of this site there are locations that have local upflows with very short *path lengths*.

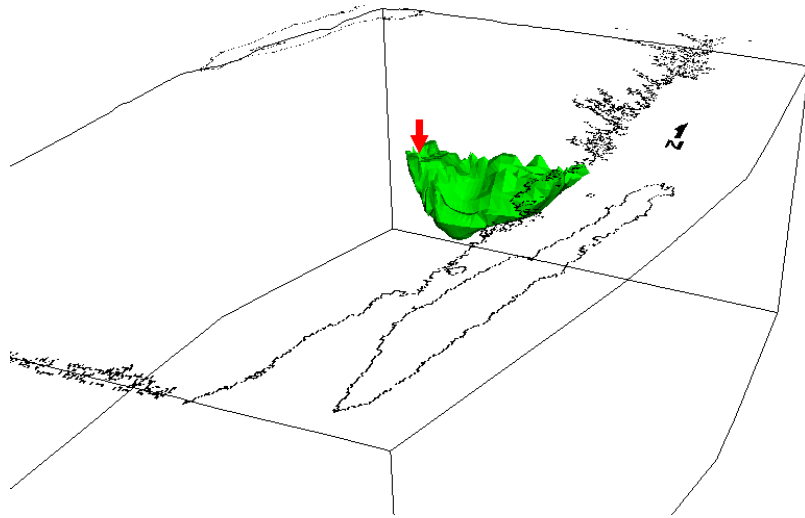
Notwithstanding the approximations inherent in visualizing flow tubes discussed above, for both extreme bedrock cases considered here, the volumes of the plumes and, thus, the *flow path volumes* are much greater for the ***Hultsfred-east*** and ***comparison*** sites than for the coastal sites. This result also is an obvious consequence of the greater *path lengths* for the inland sites where *flow path volume* ratios between long and short flow paths would tend to be at least as great as the ratios of the *path lengths* themselves.



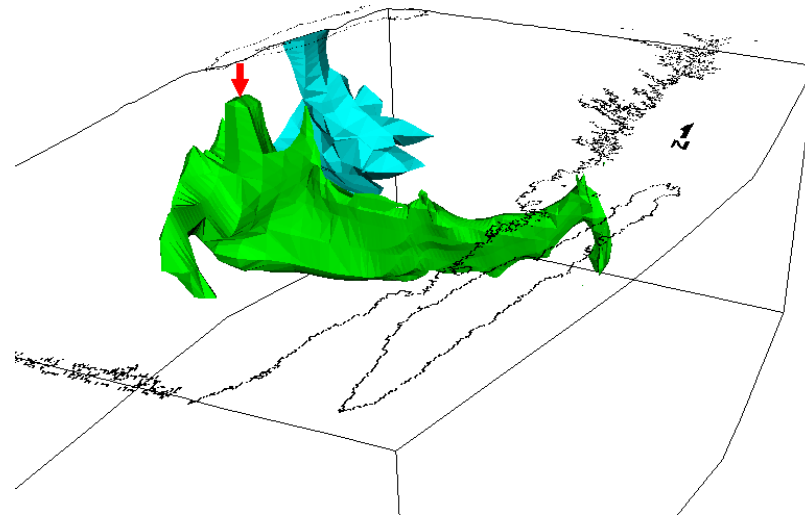
Simpevarp site. Shows $C/C_{\text{source}} > 0.50$.



Oskarshamn-south site. Shows $C/C_{\text{source}} > 0.20$.

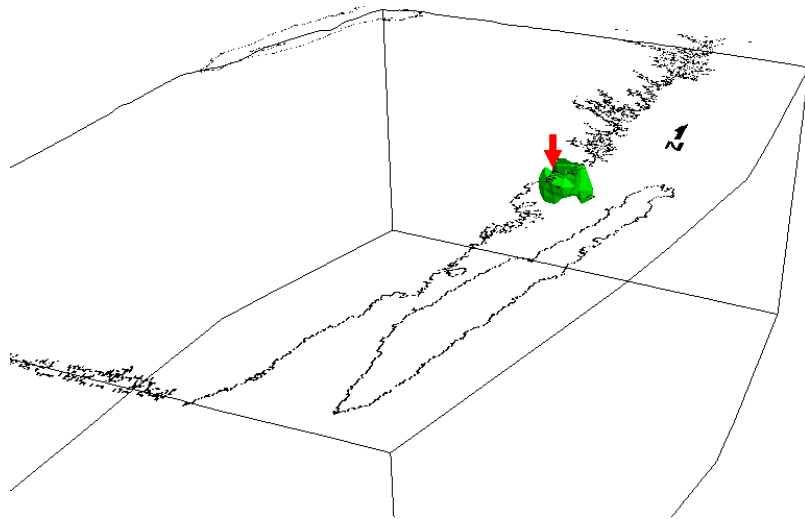


Hultsfred-east site. Shows $C/C_{\text{source}} > 0.13$.
(clarity.)

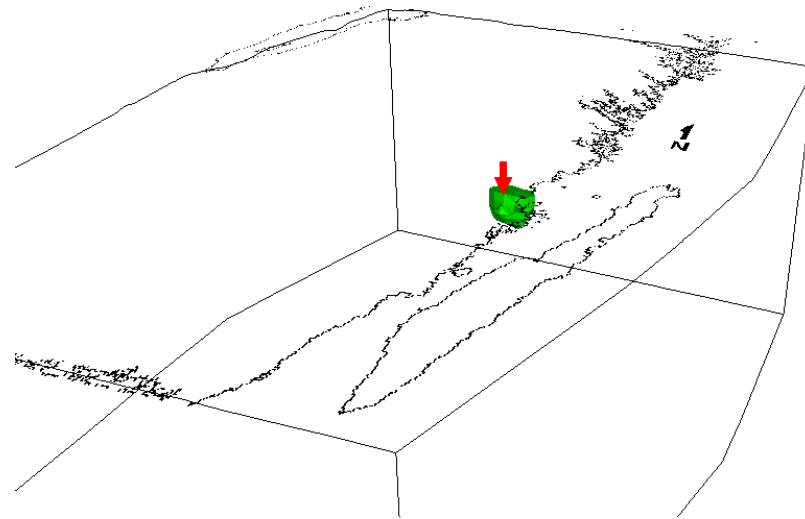


Comparison site. Shows $C/C_{\text{source}} > 0.29$. (Plume to the north shown in blue for clarity.)

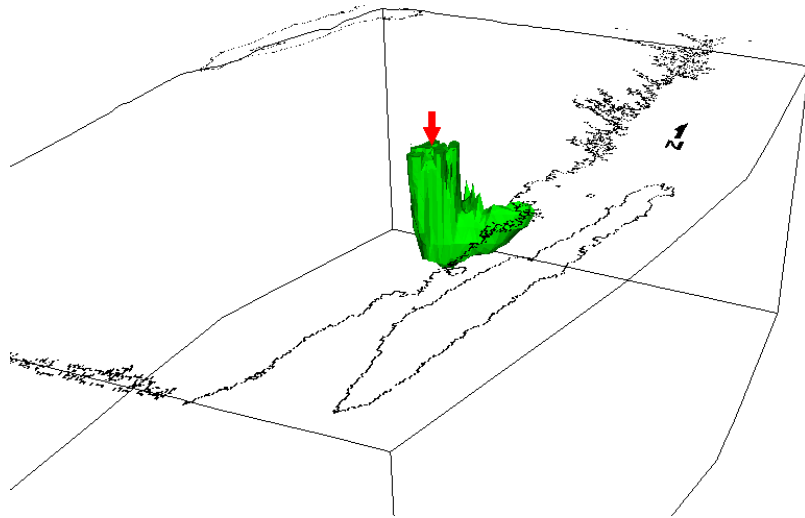
Figure 10. Flow tubes for the 10:1 Case. View from the southeast towards the northwest. Red arrows indicate repository site locations.



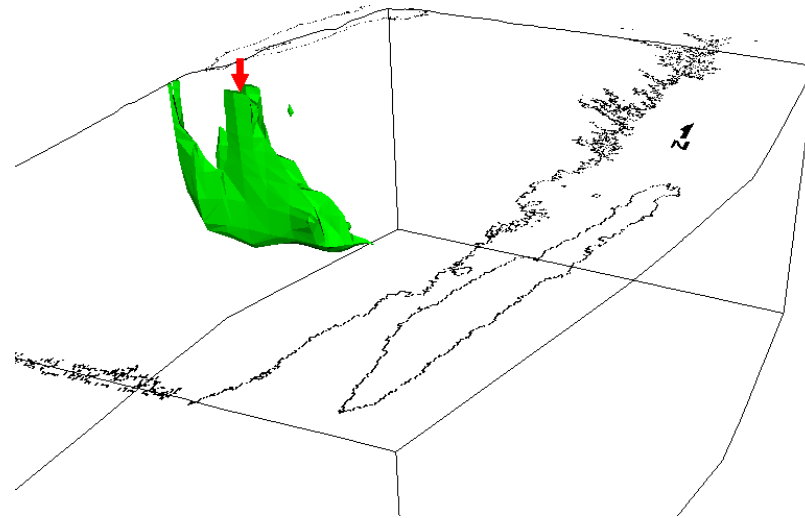
Simpevarp site. Shows $C/C_{\text{source}} > 0.30$.



Oskarshamn-south site. Shows $C/C_{\text{source}} > 0.30$.



Hultsfred-east site. Shows $C/C_{\text{source}} > 0.20$.



Comparison site. Shows $C/C_{\text{source}} > 0.35$.

Figure 11. Flow tubes for the 1:10 Case. View from the southeast towards the northwest. Red arrows indicate repository site locations.

Repository Site	Travel time (ka)				Path length (km)	
	Case (10:1)	Case (10:10)	Case (1:1)	Case (1:10)	Case (10:1)	Case (1:10)
<i>Simpevarp</i>	1 →10	1 →10	10 →100	10 → >100	< 3	< 3
<i>Oskarshamn-south</i>	1 →10	1 →10	10 →100	10 → 100	< 3	< 3
<i>Hultsfred-east (eastern part)</i>	10 →100	1 →100	10 → >100	10 → 100	25, 50	>30 non-uniform
<i>Comparison</i>	10 → >100	10 →100	10 → >100	10 → >100	60, 130	35, >45 non-uniform

Table 3. Approximate path lengths and travel times for all repository locations and bedrock cases. ‘Non-uniform’ indicates that there may be localized upflow within the repository site

Return-flow times

Maps of ***return-flow times*** at a repository depth of 500 m for each bedrock case are shown in *Figure 12*. The ***return-flow time*** is the time required for fluid beginning at any point in the 3D model domain to travel to its exit point from the domain. Thus, these maps show where, at 500 m depth, repository locations would have very high and very low *travel times* for escaping radionuclides. The ***return-flow times*** from each repository site in each of the four bedrock cases are summarized in *Table 3*. The general range of ***return-flow times*** for each site given in *Table 3* is representative of only relatively large areas within each site, and small regions of higher or lower return-flow time are ignored. The details of distributions at each site may not be over-interpreted because the numerical mesh is relatively coarse; rather, the larger patterns of ***return-flow times*** are most meaningful in these simulations.

The absolute level of ***return-flow times*** is directly proportional to the selected porosity value, but relations among ***return-flow times*** for the various cases considered are independent of the porosity. Thus, all ***return-flow times*** primarily should be considered in a sense relative to one another, as the individual values depend on the single bedrock porosity value selected for the simulations.

The ***return-flow time*** is calculated for approximate steady-state transport conditions. Because of difficulties with numerical stability for the relatively coarse mesh used when calculating a single-step steady state concentration solution (i.e., the usual technique used when the mesh is sufficiently fine), a stepwise approach was used to generate ***return-flow time*** maps at repository depth. Four special aspects of this type of simulation are listed below.

- 1- All flow is reversed in the simulation by negating the specified pressure boundary values, the gravity components, and the initial pressures (which result from the steady-state variable-density simulation). Any specified fluid sources and sinks also would have to be negated, but there were none in these simulations.
- 2- A zero-order solute source within the fluid is specified at a rate of one per year.
- 3- The SUTRA code was modified such that a concentration of zero is specified at surface nodes at which recharge occurs in the reversed flow. This corresponds to setting the ***return flow time*** to zero at discharge points in the forward flow. This helped to mitigate the effects of dispersion near the top surface of the model.
- 4- The SUTRA code was modified such that the same flow field was used throughout the transient transport simulation.

These aspects allow the flow field to be ‘timed’ such that at any inflow point, the concentration is zero, and while each parcel of fluid enters and moves through the 3D domain, its concentration increases by a value of one per year. This gives the fluid age in the model for the specified steady-state flow field. Because the flow is reversed, the concentration values resulting from this simulation represent the time it takes for flow from each point in the subsurface to reach its exit from the model – the ***return-flow time***. This ***return-flow time*** is exactly the *travel time* desired for radionuclide transport calculations in a safety analysis of a repository.

The approach to generating a nearly steady-state *return-flow time* map may be explained by example, as follows. To generate the 10 ka *return-flow time* contour, a transient simulation was run for 100 ka, and the time evolution of the 10 ka contour was observed. At times slightly greater than 10 ka, the contour changed rapidly with time as more and more parts of the ground-water-flow system reached return flow times greater than 10 ka. (In an ideal simulation free of dispersion, this change would be instantaneous.) At times much greater than 10 ka, large *return flow times* would begin to encroach upon the 10 ka contour because of dispersion. Such simulations were repeated for a range of longitudinal and transverse dispersivities. For low dispersions, an intermediate time was present during which the 10 ka contour was not changing substantially with time and was relatively insensitive to changes in the dispersivities. Results chosen from such an interval were deemed to be largely free of the effects of dispersion. To use sufficiently low dispersion, it generally was necessary to allow some localized instabilities to arise; in such areas, the *return-flow time* calculations were considered inconclusive. For each bedrock case, contours were generated in this manner for return-flow times of 0.1, 1, 10, and 100 ka. The contours were then superimposed on one another, first laying down the highest return-flow times and finally placing the lowest, to yield a complete return-flow time map. The run durations and dispersivities for each case are given in *Table 4*. The GMRES solver was used for all cases.

anisotropy case	$\log_{10}\{\text{contour level (yrs)}\}$	run duration as multiple of contour level	longitudinal dispersivities as multiple of default values	transverse dispersivities (m)
10:1	2	2.0	0.0	0.
	3	2.0	1.0	0.
	4	2.0	1.0	0.
	5	6.0	1.0	100.
10:10	2	1.6	0.0	0.
	3	4.0	1.5	150.
	4	6.0	1.0	200.
	5	6.0	2.0	200.
1:1	2	2.0	0.0	0.
	3	2.0	0.5	0.
	4	6.0	1.5	100.
	5	6.0	1.0	200.
1:10	2	2.0	0.0	0.
	3	1.4	0.0	0.
	4	6.0	2.0	200.
	5	6.0	2.0	200.

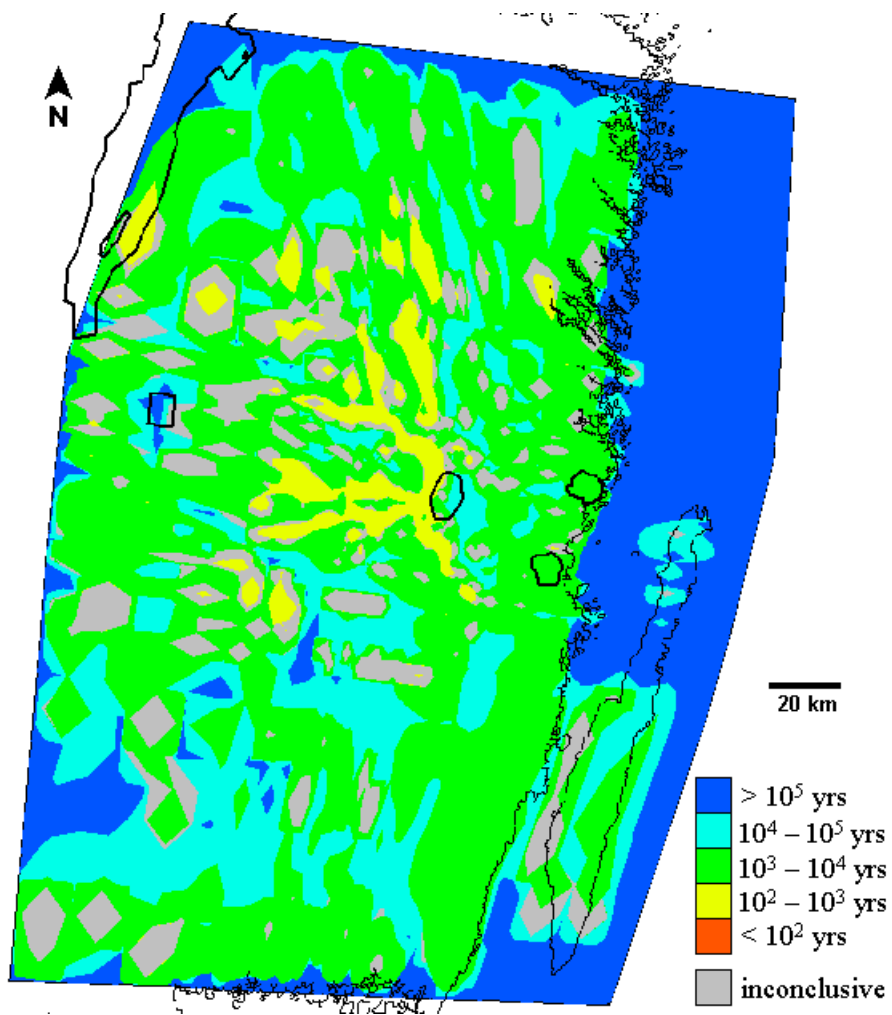
Table 4. Summary of the runs performed to create return flow time maps at repository depth. For example, to generate the 10^5 -year contour for the 10:1 case, the return flow time solution at 6.0×10^5 years was used, based on longitudinal dispersivities equal to the nominal values, and transverse dispersivities of 100 m. The nominal longitudinal dispersivities are 1/4 of the representative mesh spacing in the direction of flow; 450 - 2500 m for horizontal flow, and 62.5 - 500 m for vertical flow.

The maps in *Figure 12* show how long it would take a non-reactive (conservative) solute escaping from a repository 500 m below the land surface, at any location in southeastern Sweden, to reach its discharge point from the ground-water system. The regions indicated to be ‘inconclusive’ in *Figure 12* were affected by numerical instability and, thus, give no clear result.

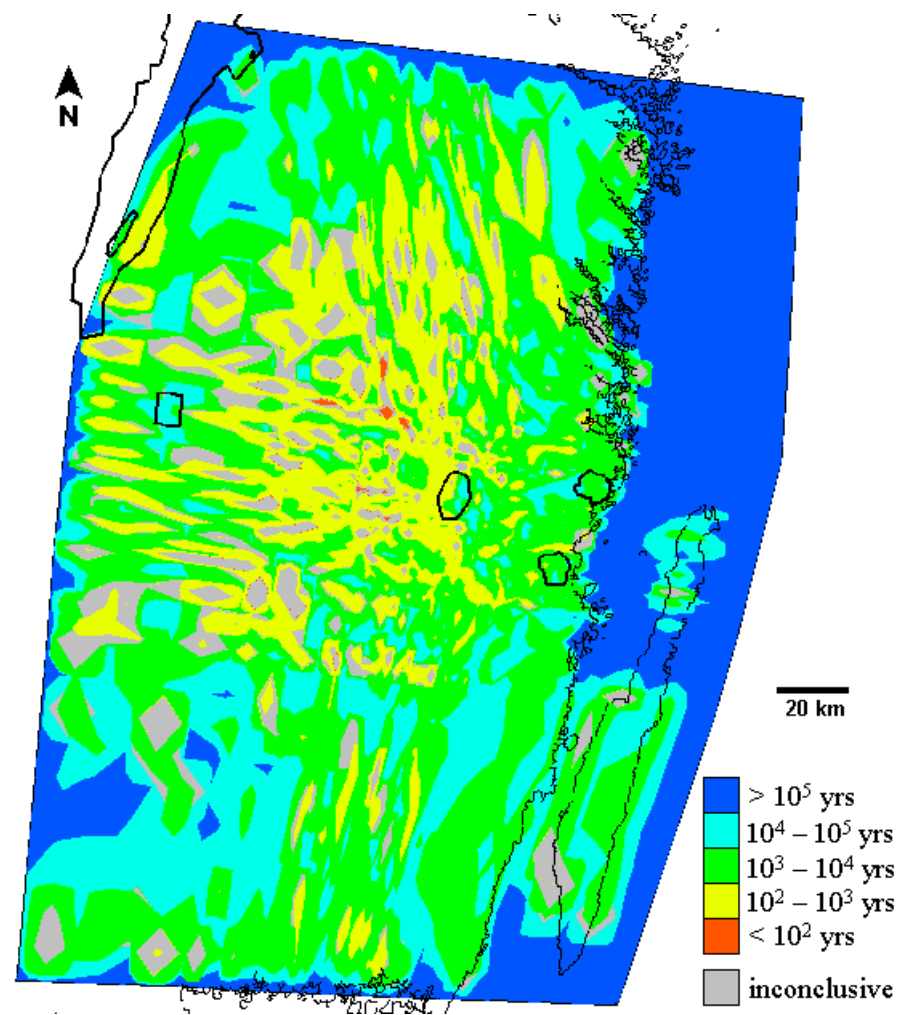
- The largest contiguous region of very low *return-flow times* is below the Baltic Sea. This results because of the very low hydraulic gradient below the sea. However, this study is restricted to consideration of only terrestrial sites.
- The lowest *return-flow times* on land, generally less than 1 ka (colored yellow in *Figure 12*), are in regions within the inland surface-water drainage network including Strångån, Silverån, and Emån streams for all bedrock cases.
- Although similar *return-flow times* occur at many locations inland, there is a continuous region of relatively low *return-flow times* along the east coast of Sweden (1 ka to 10 ka, colored green) for the simulated 10:1 case. This result probably indicates a zone of ground-water discharge from a large-scale regional flow system. In the 10:10 case, most of this coastal region is preserved, but because of the higher vertical permeability, more local topographic features are important in comparison with regional features, and shallower ground-water systems with lower *return-flow times* penetrate to repository depth, the continuity of the coastal band is not as complete. The 1:1 case and 1:10 case exhibit nearly the same pattern as the 10:10 case, but with 10 times longer *return-flow times* than for the 10:10 case. For the case in which the vertical permeability is higher than the horizontal (1:10), the flow field at 500 m depth is even more dominated by local topographic variations and local flow cells. Indeed, the coastal area is topographically smoother than inland areas (to some degree because the finite elements are larger near the coast than farther inland – see last point below), and it exhibits relatively high *return-flow times* in comparison with inland locations for this case.
- The coastal repository areas at *Simpevarp* and *Oskarshamn-south*, both have *return flow times* of less than 10 ka for each bedrock case with high horizontal permeability. For the cases with low horizontal permeability, *return flow times* at *Simpevarp* and *Oskarshamn-south* are approximately 10 times greater than with high horizontal permeability.
- The eastern portion of the repository area at *Hultsfred-east* has a return flow time of up to 100 ka for each bedrock case with high horizontal permeability (10:1 and 10:10). This time is up to 10 times greater than at the near-coastal sites. Whereas there is some spatial variation of *return-flow time* in the *Hultsfred-east* area, the eastern portion of this area persistently has a relatively high value in both of these cases, and may be of sufficient size to guarantee relatively long travel times for a repository located there. For the isotropic case with low horizontal permeability (1:1), return flow times in parts of the eastern portion of *Hultsfred-east* are greater than 100 ka, greater than at the coastal sites. For the case with lower horizontal than vertical permeability (1:10), return flow times for the eastern portion of *Hultsfred-east*, and for *Simpevarp* and *Oskarshamn-south* are similar.

- A *comparison* site, at a location arbitrarily selected for relatively good performance in all bedrock cases, consistently exhibits very high *return-flow times* from 10 ka to greater than 100 ka (except with a maximum of 100 ka for the isotropic, high-permeability case, 10:10). Furthermore, the advantageous area is relatively large, facilitating the siting of a repository reliably within the area of high *return flow time*.
- In areas where mesh discretization is coarsest (for example, in the southwestern portion of the model area), *return-flow times* tend to be higher at least in part because it is not possible to represent relatively high local-scale topographic gradients in these areas. A uniformly discretized mesh would avoid these areal discrepancies, and present results for *return-flow times* must be interpreted considering this limitation.

In each bedrock case, there are regions in southeastern Sweden with very high *return-flow times*. Some of these regions have relatively high *return-flow times* for all bedrock cases considered. These regions include the eastern portion of *Hultsfred-east* and the *comparison* sites. Other regions have *return-flow times* that vary substantially among the bedrock cases considered. These include the coastal sites, *Simpevarp* and *Oskarshamn-south*.

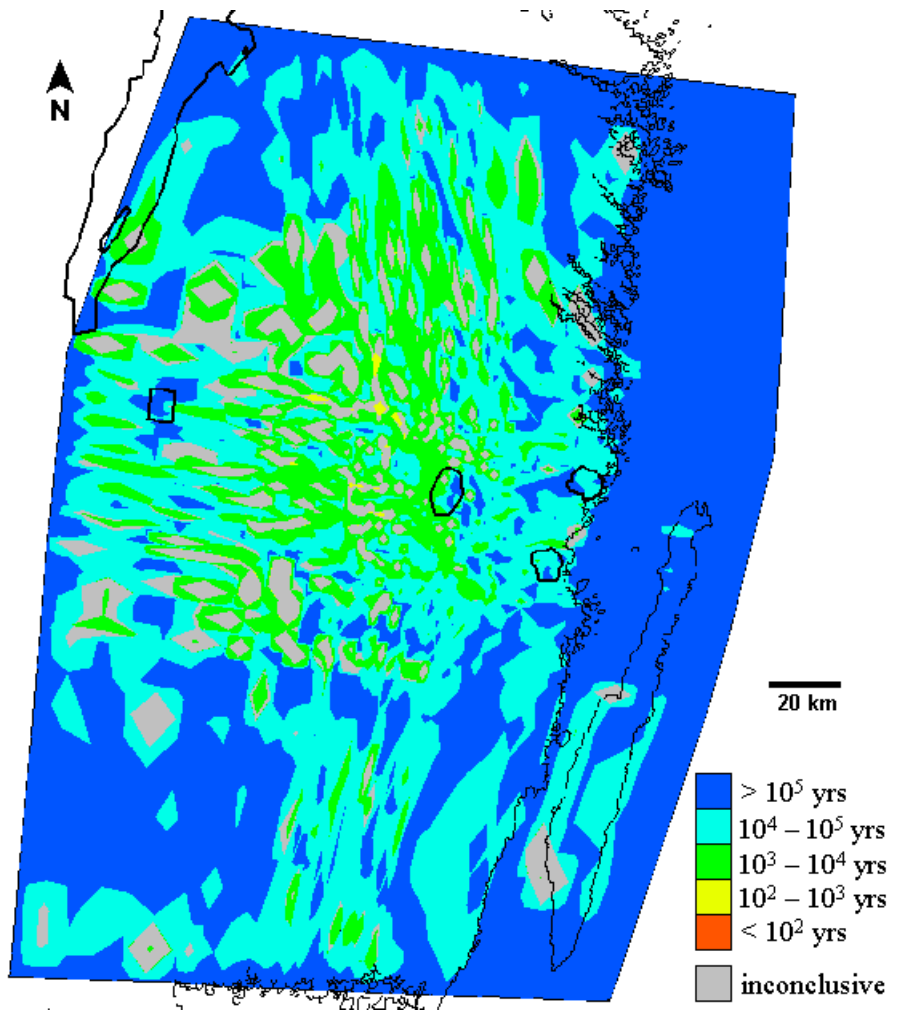


10:1

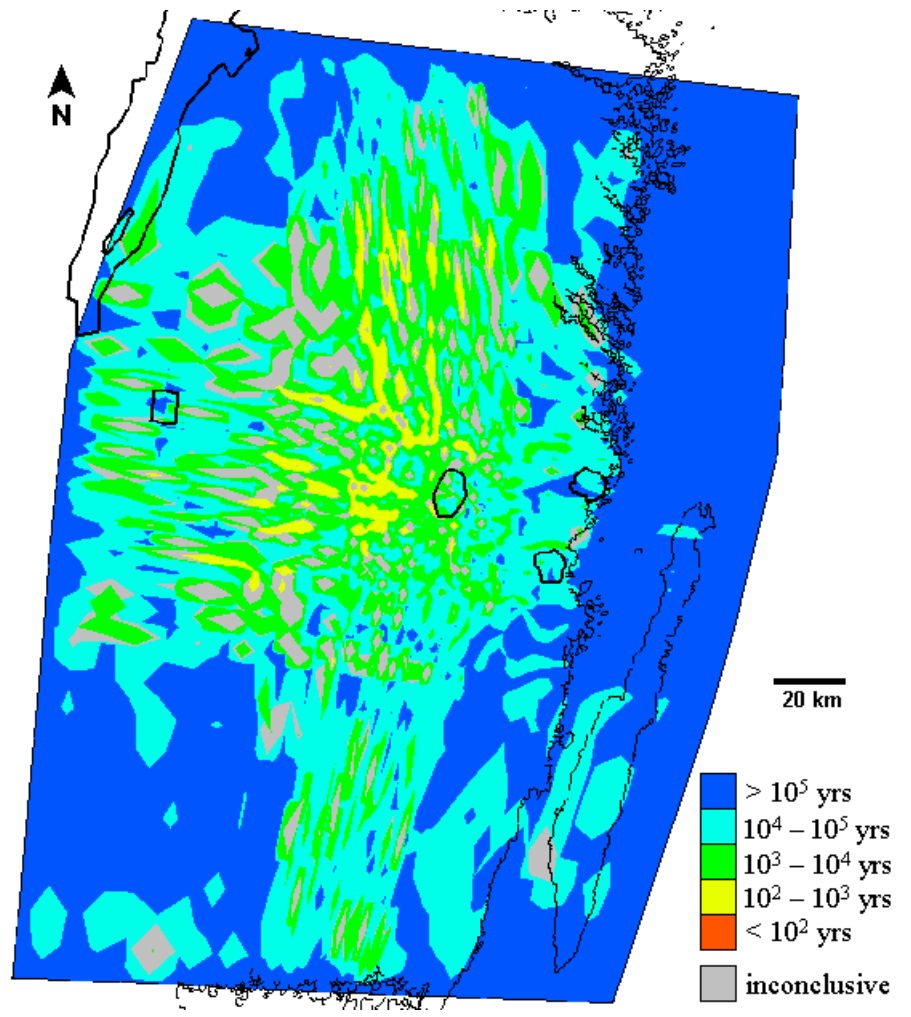


10:10

Figure 12.



1:1



1:10

Figure 12, continued. Return flow times at depth of 500 m for four bedrock permeability cases (*horizontal permeability: vertical permeability*). Map view with four repository sites indicated.

Recharge and discharge areas

The recharge and discharge areas for ground water at the land surface in southeastern Sweden, as determined by the 3D simulation, are superimposed on a raised relief map of the region (for the bedrock Case 10:1) (*Figure 13*). Areas of discharge occur along stream valleys and in most depressions in the terrain, whereas relatively high ‘plateaus’ are usually recharge areas.

Maps of the recharge and discharge areas for each bedrock case are shown in *Figure 14*. The locations of recharge and discharge areas do not change substantially as a result of the different assumptions for bedrock properties. Whereas the presence of high *return-flow times* at 500 m depth (repository depth) depends only partly on whether the surface above is in a recharge area, location in a recharge area may be a necessary condition for high *return-flow times*.

It is clear that for the 10:1, 10:10 and 1:1 bedrock cases, the coastal repository areas, *Simpevarp* and *Oskarshamn-south*, are below primarily discharge areas for ground water, whereas a recharge area is above most of the *Hultsfred-east* site. The *comparison* site is entirely below a recharge area. For the 1:10 case, recharge and discharge locations at the land surface change rapidly from place to place, and all sites contain both recharge and discharge areas at the surface.

The average recharge and discharge rates for each bedrock case are given in *Table 5*. Freshwater recharge in cm/yr was computed by dividing the total fresh recharge (kg/s) by one-half of the “non-sea” area and making the appropriate unit conversions. A calculation of surface areas in the model result determined that recharge areas and discharge areas each cover very close to one half of the land surface of southeastern Sweden. Thus, in each model, average recharge rates to the southeastern Sweden bedrock ground-water system could be computed by dividing the total freshwater recharge by one-half of the entire land area in the model.

Model simulated recharge rates are much lower than expected for total recharge in a humid climate, but the rates given refer only to recharge of deeper flow systems within the bedrock. Recharge and discharge in the shallow subsurface (e.g., within the soil zone), where flow rates usually are very high, may be much greater, but the present model does not resolve this shallow recharge. Thus, the rates given in *Table 5* represent recharge to the deep bedrock, not to surficial aquifers.

Note that the model cannot take into account recharge to (and discharge from) flow systems that are smaller than the spatial resolution of the water-table topography specified in the model. Thus, what is meant by an average ‘recharge rate’ depends on the spatial resolution of the modeled ground-water system. The recharge calculated from a finely discretized model would include inflows for smaller flow cells (located in the shallow subsurface) than in a more coarsely discretized model. Recharge in a finely discretized model is therefore larger than in a more coarsely discretized model. Of course, regardless of the model resolution, the total discharge equals the total recharge in steady-state flow.

These recharge-discharge area results are derived from simulation of transport with a very long time step to approximate steady-state transport conditions for the situation where the flow field is the same as the steady-state result of the variable density simulation. SUTRA was modified such that the concentration was specified to be zero at recharge nodes and one at discharge nodes (based on steady-state fluxes). First-order decay was used to prevent ‘solute’ from penetrating too deep in the model domain, thereby avoiding numerical stability problems in the simulations. Based on the resulting concentrations at the surface, recharge nodes were colored blue and the discharge nodes were colored yellow (*Figure 13*).

Case	Total flows in kg/s			Freshwater recharge rate in cm/yr (Computed using one-half of “non-sea” area)
	Freshwater recharge	All recharge	All discharge	
10:1	213.1	214.4	214.0	0.034
10:10	505.6	508.1	507.4	0.080
1:1	47.4	47.7	47.5	0.0076
1:10	88.0	88.4	88.2	0.0140

Table 5. Average recharge to the 3D model for each bedrock case.

Non-sea area = $3.94e+10 \text{ m}^2$, Sea area = $0.98e+10 \text{ m}^2$,

Total model domain area = $4.92e+10 \text{ m}^2$.

Differences between modelled recharge and discharge values are due to model solution error.

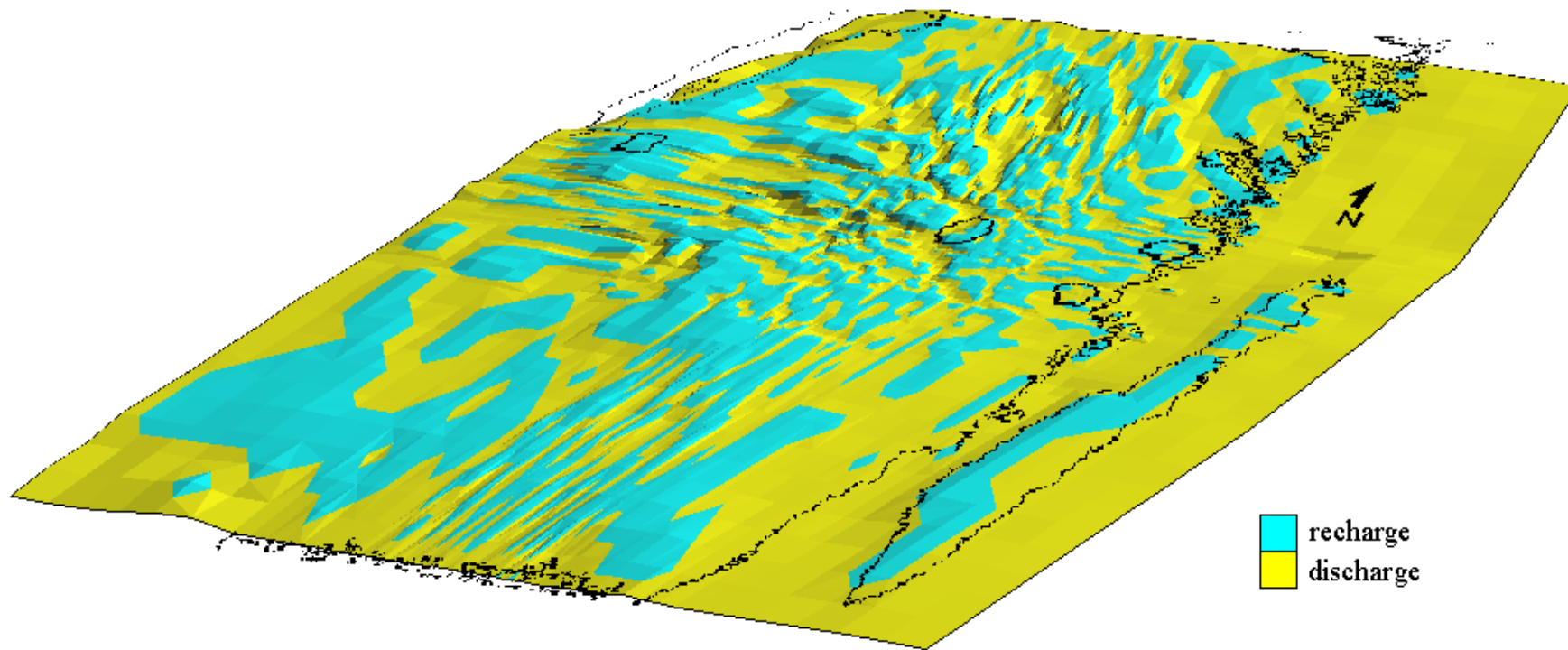
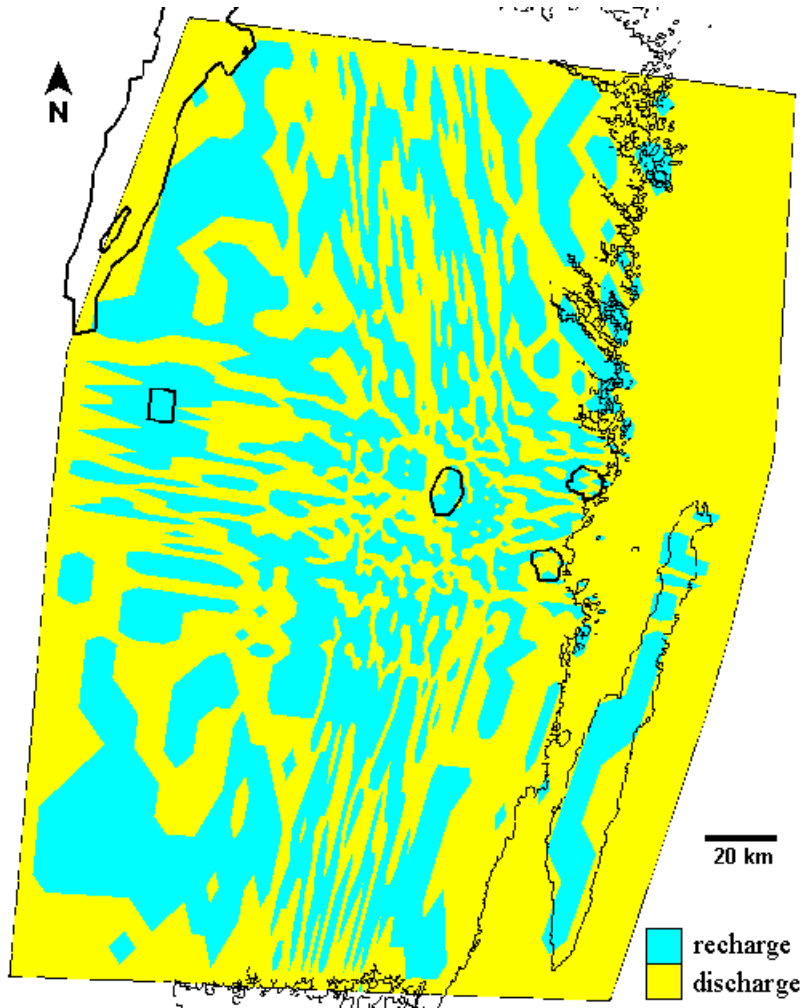
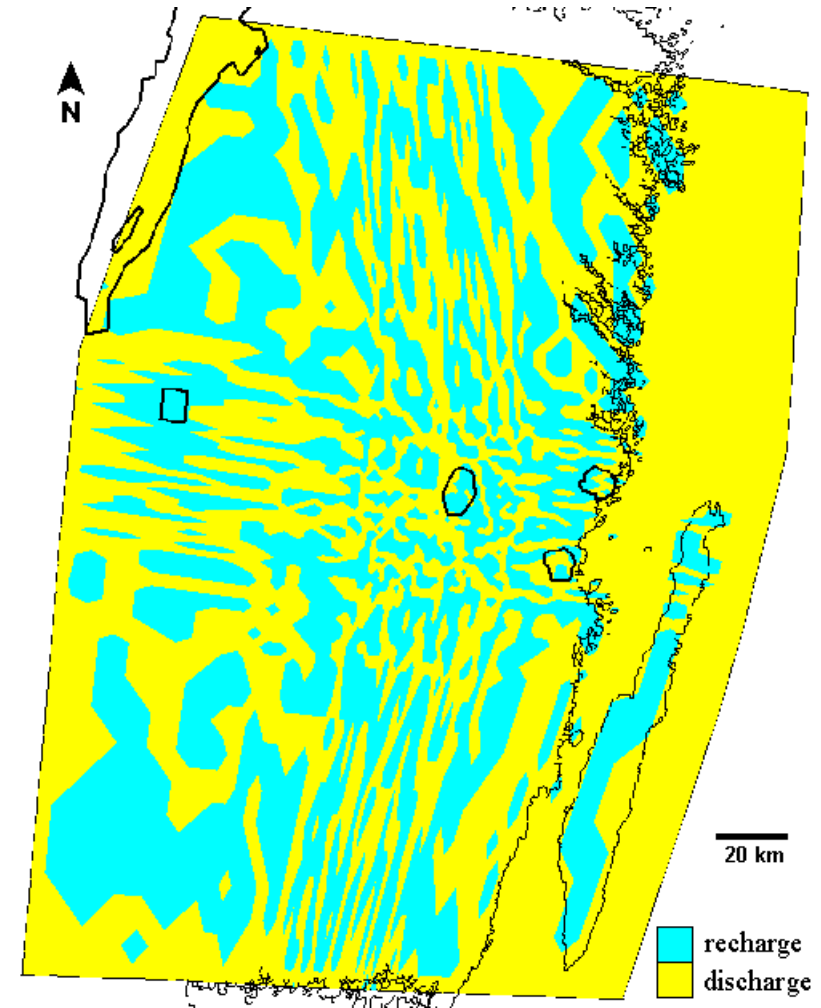


Figure 13. Recharge and discharge areas for the **10:1 Case** at land surface. Oblique view from southeast towards northwest. Vertical exaggeration = 20x. Map shows the coastline, the shoreline of Vättern Lake, and the proposed repository siting areas.

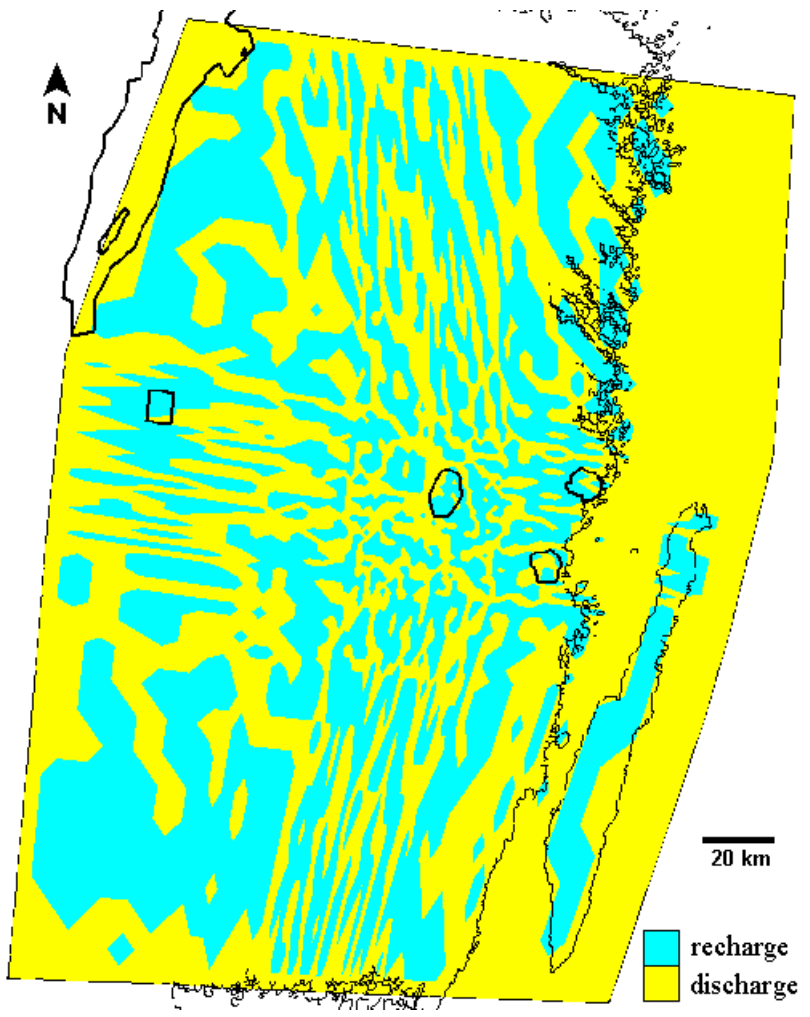


10:1

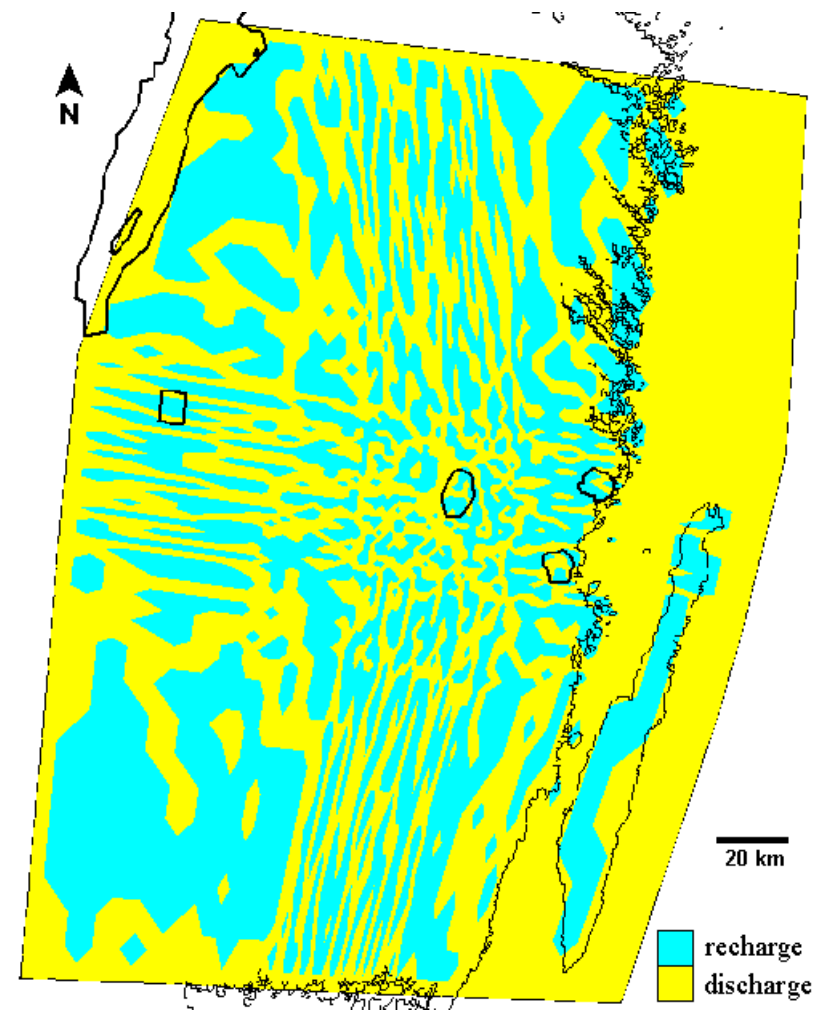


10:10

Figure 14.



1:1



1:10

Figure 14, continued. Map of recharge and discharge areas at land surface. Map view with four repository sites indicated. (Blue = recharge area. Yellow = discharge area.)

DISCUSSION OF 3D RESULTS

The present study provides specific results concerning the ground-water flow system in southeastern Sweden, but should be considered primarily as a demonstration of techniques and concepts related to recharge-area siting of nuclear waste repositories. Because of the relatively coarse discretization used in order to allow simulations to be carried on a desktop computer, the present results should not be considered as definitive. The effect of the coarse mesh on the results is apparent in most maps, as patterns of color tend to be parallel to, and some undulations equivalent in frequency to, the finite-element mesh discretization in each area. More refined hydrogeologic characterization of the region and use of a much finer mesh with more regular areal discretization on a larger computer would allow numerical solutions to be more uniformly accurate.

All modeling results for path length and travel time for all for repository locations considered and for all bedrock permeability cases considered are summarized in *Table 3*.

- *Path lengths* from the near-coast repository areas, *Simpevarp* and *Oskarshamn-south*, are at least 10 times shorter than for inland locations, *Hultsfred-east*, and the *comparison* site for the extreme bedrock cases considered. Coastal sites probably are within a band of short *path length* (less than 3 km) from a repository.
- *Flow path volumes* from the near-coast repository areas, *Simpevarp* and *Oskarshamn-south*, are at least 10 times smaller and may be a few orders of magnitude smaller than for inland locations, *Hultsfred-east* and the *comparison* site for the extreme bedrock cases considered. (Volumes provided by the simulations were not measured exactly but relations among volumes for different sites may be determined by inspection of the figures showing flow paths.)
- *Travel times* (i.e. *return-flow times*) from the near-coast repository areas, *Simpevarp* and *Oskarshamn-south*, generally are 10 or more times shorter than for inland locations, *Hultsfred-east*, and the *comparison* site for cases with the high value of horizontal hydraulic conductivity of the bedrock (10:1 and 10:10). For the cases with low horizontal values of bedrock conductivity, *travel times* are similar among all sites. Travel time is a function of both path length and average flow velocity. The travel times, computed for the four sites based on four different anisotropic bedrock cases, reflect the interacting influences of regional-scale topography, local-scale topography, and variable-density ground-water flow.
- For the *Hultsfred-east* area, the eastern portion has much longer *travel times* than the western portion. The eastern portion may be large enough to contain a repository within the region of longer travel times. However, for the case with higher vertical than horizontal conductivity (1:10), there are patchy locations within the site with upwards flow and, thus, short *flow paths* and *travel times*. Within the low resolution of the 3D model, it

is not possible to determine the detailed flow pattern within any site, so the areal continuity of downflow within the eastern portion of *Hultsfred-east* cannot be verified here.

- The *comparison* site is one of the better sites in southeastern Sweden considering *path length* and *travel time*, as well as *flow path volume*. At the edges of the site for the case with higher vertical than horizontal conductivity (1:10), there are small regions with upwards flow and thus short *flow paths* and *travel times*. Again, their precise distribution is not certain because of the low spatial resolution of the model. The *comparison* site is highlighted in this report not to suggest that a repository be located here, but to point out that it is possible to identify sites in southeastern Sweden that have extremely high values of the *path length* and *travel time*. These are among the most important hydrogeologic factors that determine the long-term radiological safety of a high-level nuclear waste repository.

The inland sites have longer *path length* than the coastal sites for all of the bedrock models. Furthermore, inland sites *travel times* are longer than the coastal sites for some of the bedrock cases, and are about equivalent for the other cases. The *comparison* and eastern portion of the *Hultsfred-east* sites may be preferable as repository sites, in terms of the hydrogeologic factors considered, to the coastal sites at *Simpevarp* and *Oskarshamn-south*.

Furthermore, an examination of the flow fields in the two extreme cases of bedrock anisotropy considered (*Figure 9*) indicates that, in both cases, the *Comparison* site and *Hultsfred-east* site are located in areas of recharge and deep downflow, while the coastal *Simpevarp* and *Oskarshamn-south* sites are located in areas of shallow flow and discharge. If the bedrock anisotropy considered includes the regional anisotropy that actually represents the bedrock in the region, then the inland sites are indeed located within two of the primary recharge areas in the region.

Conclusions

A general approach to identification of nuclear waste repository locations that maximize key safety hydrogeologic safety factors, *path length*, *travel time* and *path volume*, involves the use of a set of 3D variable-density numerical models of the subsurface, each representing a different conceptual model of regional bedrock properties. The need to use many conceptual models is an admission that it is not possible to define a unique model of the bedrock because its properties at the spatial scales of interest cannot be easily measured. Indeed, it is likely that the hydrogeologic properties of Sweden's bedrock cannot be well-enough known at the range of scales from small to large required for definitive analysis of ground-water flow with respect to nuclear waste repository safety based on a single 'correct' description. For example, whereas regional flow fields are described in this report, it is possible that a high local topographic gradient may combine with one or more possible local fracture zones to create, in effect, an extremely high local anisotropy that, acting like a set of closed pipes, would create a local flow field near a repository with little relation to the regional flow fields. In this case, a combined discrete-fracture/continuum model that can include possible local structures as well as the regional fabric should be selected as one among the number of cases tested. The general approach to identifying 'upstream' sites thus involves identification of sites that perform well in all possible conceptual models of the bedrock. If the bedrock models are well selected and represent the full range of possibilities, a repository placed at a site that performs well for all models is most likely to perform well in the reality of the actual site properties.

In order to improve chances that a repository site will maximize the safety-related factors, *path length*, *travel time* and *path volume*, the following siting objectives need to be met:

- The repository should be located below a ground-water recharge area.
- The repository should be located far from potential discharge areas for the site.
- The repository should be located in a contiguous region of high *return-flow time*, large-enough to include the repository.
- The repository should be located in region where ground-water flow is downward for considerable (a few kilometers) depth below the repository.

The above objectives need to be met for all or most of the possible conceptual models of the bedrock that may describe the site and the region.

Also, some related siting objectives are:

- The repository should be located as deep as possible to avoid surface flow cells.
- The repository should be located in a region with as smooth as possible topography to avoid locating below a region with substantial surface flow systems.

It should be noted that for a repository located according to these objectives, in a recharge area with generally downward ground-water flow, there is less need for detailed characterization of the structural properties of the site for purposes of determining hydrogeological safety factors.

For southeastern Sweden, some broad conclusions may be drawn about ground-water flow given the range of bedrock properties considered here. If the horizontal permeability of the bedrock is

relatively high in comparison with the vertical permeability, then regional systems dominate ground-water flow at many inland locations at repository depth (500 m). Selection of a repository site within a recharge area for these flow systems would provide large *path lengths*, *travel times* and *path volumes*. This situation describes the classic case of regional flow systems discharging at the coast, causing coastal repositories to have relatively lower *path lengths*, *travel times* and *path volumes* than inland recharge-area repositories. It is possible that if there are shallow topographic gradients near the coast, these conditions partly offset the disadvantage of short path lengths near the coast (possibly, for example, at potential coastal repository sites in Uppland, Sweden).

If the vertical permeability of the bedrock is relatively high in comparison with the horizontal permeability, then small local ground-water systems would tend to dominate at repository depth (500 m) at most locations. This situation makes *travel times* similar at both the inland and near-coastal sites considered. Despite this situation, the *path lengths* and *path volumes* at the inland sites considered are found to be substantially larger for all bedrock cases considered. If the bedrock in Sweden has higher vertical than horizontal permeability, then the potential safety advantage of the inland sites would be in providing larger *path lengths* and *path volumes* but not necessarily longer *travel times*.

Sites are present in southeastern Sweden with relatively high *return-flow times*, irrespective of the bedrock permeability anisotropy. Inland areas are present that are ‘windows’ for recharge to the deeper ground-water systems; the locations of these ‘windows’ in southeastern Sweden seem to be rather independent of the bedrock permeability. Recharge in these areas travels downward far below repository depth before joining a deep regional ground-water flow path in a large-scale flow system, and discharge eventually is only at distant points. Selection of sites within ‘windows’ to the deep flow system, even in the case of higher vertical than horizontal permeability, could take advantage of the variable-density flow system wherein deep brines tend to confine some flows near the surface, but recharge that enters the deep brine system has extremely long *travel times* (over 10^5 years). These ‘windows’ would make ideal repository sites that maximize *path length*, *travel time* and *path volume* for potentially escaping radionuclides.

Near-coastal sites may be among the less-desirable choices possible in the region in terms of these hydrogeologic safety factors, because the coast is a discharge area for a ground-water flow system of some scale, either regional or local. This result is true irrespective of the properties of the bedrock. In terms of the long-term repository safety provided by hydrogeologic barriers, inland sites that can be identified as being below recharge areas at upstream positions in regional ground-water flow systems may be better bets for safety than near-coastal sites, maximizing *path length*, *path volume* and possibly *travel time* for any escaping radionuclides.

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